## North Carolina Monroe Expressway

 Traffic and Toll Revenue Study
## Final Report



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## Chapter 1

## Introduction

This report documents the methodology and findings of the Monroe Expressway Traffic and Revenue Study conducted for the North Carolina Department of Transportation (NCDOT) and the North Carolina Turnpike Authority (NCTA). The purpose of the study is to develop a forty-year annual traffic and toll revenue forecast for the proposed Monroe Expressway (the Project) from its assumed opening year (2019) through 2059. The traffic and toll revenue forecasts are suitable for use in support of bond financing.

### 1.1 General Project Description

The proposed Monroe Expressway, shown in Figure 1.1, will be a controlled-access toll road, roughly parallel with US 74, extending from US 74 near I-485 in Mecklenburg County to US 74 between the towns of Wingate and Marshville in Union County, a distance of approximately 19.7 miles.

US 74 is a major east-west roadway that connects southeastern North Carolina, including the Port of Wilmington, to the Charlotte metropolitan area and points beyond. US 74 is a primary transportation corridor between Union County and Charlotte/Mecklenburg County. In addition, US 74 provides access to many retail, commercial and employment centers. Due to its important regional and local roles, US 74 traffic volumes have increased and traffic congestion occurs during weekday peak time periods. The Monroe Expressway would provide a high-speed alternative to US 74 for area motorists.

### 1.2 Project Alignment and Toll Concept

Figure 1.1 shows the general alignment of the proposed Monroe Expressway and the toll concept. The project alignment will follow the existing US 74 for approximately one mile from east of I-485 to the east side of Stallings Road (SR 1365) and continue on a new southeasterly alignment to the terminus with US 74 between the towns of Wingate and Marshville. The project is almost entirely contained in Union County, NC, although the westernmost portion is located in Mecklenburg County.

Six intermediate interchanges will be located at Indian Trail Fairview Road (SR 1520), Unionville Indian Trail Road (SR 1367), North Rocky River Road (SR 1514), US 601, NC 200 (Morgan Mill Road), and Austin Chaney Road (SR 1758). These will be full-access interchanges. Partial interchanges will be located between US 74 and the Monroe Expressway at the east and west termini of the tolled portion of the project. Additionally, access to and from Stallings Road and McKee Road will be available via frontage roads in the toll free portion of the project.

Tolls will be collected electronically via overhead mainline gantries using both electronic toll collection (ETC) and video toll collection (VTC). The NCTA VTC program is named Bill by Mail (BBM). Cash payments on the roadside will not be available. A gantry will be located on each mainline section at appropriate locations. Gantry locations shown in Figure 1.1 do not represent exact locations. There will not be any toll-free movements on the project from the western interchange with US 74 to the eastern interchange with US 74. Toll rates would be based on the distance covered on each mainline section. Each mainline section with a gantry is called a toll zone. The toll zones are numbered 1 through 7, and are depicted in Figure 1.1 along with the distance covered for each toll zone.

NCDOT Monroe Expressway Traffic and Toll Revenue Study


PROJECT ALIGNMENT AND TOLL CONCEPT

The project includes an upgrading of US 74 for approximately one mile at the far western end, to a controlled-access roadway with one-way frontage roads. The inset box in Figure 1.1 shows the upgraded section of US 74, including the frontage road system and the partial interchange with the Monroe Expressway. There will not be any toll collection on the upgraded section of US 74 or the frontage roads. A detailed description of the project configuration is provided in Section 3.3.

### 1.3 Scope of Work

The scope of work for this study was designed to produce traffic and toll revenue forecasts suitable for bond financing. The study work scope is comprised of the following seven tasks:

- Task 1: Data Collection and Summarization
- Task 2: Corridor Growth Analysis
- Task 3: Model Refinement and Calibration
- Task 4: Traffic and Toll Revenue Analysis
- Task 5: Traffic and Toll Revenue Sensitivity Tests
- Task 6: Study Documentation and Coordination
- Task 7: Traffic and Revenue Sensitivity Tests

A brief description of these tasks is provided below.

## Task 1: Data Collection and Summarization

## Subtask 1.1: Traffic Counts

- CDM Smith obtained available traffic count data in the study area from the NCDOT.
- Seven-day vehicle classification counts were conducted at thirteen locations in the study area.


## Subtask 1.2: Video License Plate Recognition

An automatic license plate recognition (ALPR) survey was conducted at 13 locations in the study corridor from 5:30 to 7:00 PM on a weekday in October 2015. Seven locations were on US 74, two locations were on Old Charlotte Highway, and three locations were on Secrest Short Cut Road. License plate images were obtained by video cameras in the westbound direction of travel. The survey was conducted to aid in identifying trip distances in the study area.

## Subtask 1.3: Travel Time and Speed Data

- INRIX travel time data was obtained from RITIS.org for US 74 and other roadways in the study area.
- CDM Smith conducted independent travel time runs to validate the INRIX data.


## Subtask 1.4: Transportation Improvements

Information on current and planned roadway improvements in the study area was collected from the NCDOT and various planning agencies.

## Task 2: Corridor Growth Analysis

Economic growth forecasts are one of the most critical elements of any traffic and revenue forecast, particularly for a new toll facility such as the Monroe Expressway. For this study, an independent economist, Dr. Stephen J. Appold, was engaged to review the socioeconomic and land-use forecasts that are assumed in the travel demand model. The goal of this effort was to evaluate the reasonableness of the socioeconomic and land-use assumptions and make suggested adjustments where appropriate. Dr. Appold revised population and number-of-households in select traffic analysis zones (TAZs) in the study area. After a review of Dr. Appold's work, CDM Smith implemented the recommended changes by developing new trip tables based on the new socioeconomic data.

## Task 3: Model Refinement and Calibration

CDM Smith obtained the most recent travel demand model, the Metrolina Regional Model 2015 version 1.1 (referred to as MRM or MRM15v1.1 in this report). This model is a key toll for evaluating future travel demand in the Charlotte/Mecklenburg County area. A review of the networks was conducted, including a review of the size of the TAZs in the study area for potential disaggregation and evaluating whether additional roads needed to be added to the networks.

The model was calibrated to base year 2015 conditions in the immediate project area. The traffic assignments were calibrated to collected traffic counts, travel speeds, and trip distance data. Calibration refinements were carried through to future-year assignments.

## Task 4: Traffic and Toll Revenue Analysis

The refined model was used to run a series of traffic assignments. Each assignment was run for four time periods: AM Peak, Midday, PM Peak and Overnight. Toll sensitivity assignments were conducted at opening year (2019) and 2030 to determine optimum per-mile toll rates for the project. Optimum rates were developed for each future-year traffic and revenue assignment. The MRM15v1.1 supports model years 2015, 2025, 2030 and 2040. CDM Smith developed a 2019 trip table and network to reflect opening year conditions.

Future-year assignments were conducted with the selected toll rates to evaluate the traffic and toll revenue potential of the Monroe Expressway. Based on the traffic modeling analysis, annual estimates of traffic and toll revenue were developed for the base-case condition from 2019 through 2040. The forecasts beyond 2040 were developed by assuming a modest increase in traffic growth on the project.

Toll revenue estimates in the early years were adjusted downwards to reflect ramp-up; the pattern of gradual build-up in demand for a new road. Finally, estimates of toll revenue leakage were developed to reflect the loss of toll revenue associated with video tolling, such as unreadable license plates, unidentified vehicle owners, and account collection issues.

## Task 5: Traffic and Toll Revenue Sensitivity Tests

A series of sensitivity tests were performed to identify how sensitive the base-case traffic and toll revenue estimates are to changes in specific variables. These variables include:

- Reduce economic growth by 30 percent,
- Reduce motorist values of time by 25 percent,
- Increased ETC market share,
- Increased motor fuel prices, and
- Reduced truck share.


## Task 6: Documentation and Coordination

CDM Smith produced a series of technical memoranda to document key points in the study. These memoranda were submitted to the NCDOT/NCTA for review and comment prior to advancing to the next stage of the study. These technical memoranda included:

- Monroe Expressway - Data Collection, dated December 18, 2015,
- Monroe Expressway - Independent Economic Review, dated December 30, 2015,
- Monroe Expressway - Project Configuration and Modeling Inputs, dated February 3, 2016, and
- Monroe Expressway - Toll Rate Assumptions, dated March 10, 2016.

In addition, CDM Smith submitted Dr. Appold's report to the NCDOT. The report, titled Evaluation of the Socio-economic Estimates Underlying the Study of the Feasibility of the Proposed Monroe Expressway, dated February 17, 2016, presented Dr. Appold's methodology and findings.

The documents just described are summarized in this report in corresponding chapters, as appropriate.

### 1.4 Report Structure

This report consists of seven chapters.

- Chapter 1: Introduction - contains the purpose of the study, a description of the project, and the structure of the report.
- Chapter 2: Existing Conditions and Data Collection - presents the data collection efforts and the traffic conditions in the immediate study corridor.
- Chapter 3: Network Refinement - summarizes changes made to the base year and future year networks.
- Chapter 4: Independent Economic Review - summarizes the socio-economic assumptions in the MRM, the review of the socio-economic data by the economist, and the revisions to the socio-economics based on the economist's recommendations.
- Chapter 5: Model Calibration - reviews the model calibration methodology. Provides data to illustrate the calibration in the study area.
- Chapter 6: Traffic and Toll Revenue Analysis - describes the modeling inputs and process, the toll sensitivity analysis, the traffic and gross toll revenue forecast for the Monroe Expressway, and the gross toll revenue forecast adjusted for revenue leakage and fee revenue.
- Chapter 7: Traffic and Revenue Sensitivity Tests - describes the results of the tests.


## Chapter 2

## Existing Conditions and Data Collection

This chapter summarizes the data collection efforts undertaken as part of this study, as well as the existing traffic and travel conditions they are meant to reflect. The following sections will discuss traffic volumes, travel speeds, and travel patterns in the project corridor. This data was used to aid in model calibration as well as for use in developing traffic and toll revenue annualization factors since the model reflects a typical weekday. In addition to the information provided in this chapter, a detailed Technical Memorandum titled NCDOT TIP R-3329/R-2559 Monroe Expressway - Data
Collection was prepared and submitted to NCDOT in December 2015.

### 2.1 Data Collection Program

CDM Smith collected the following data for use in this traffic and toll revenue study:

- Available average annual daily traffic (AADT) counts were obtained from the NCDOT.
- Vehicle classification counts were conducted by The Traffic Group, a sub-consultant to CDM Smith, at 15 locations on US 74 and several other roads that are alternatives to US 74. The traffic counts were conducted at all the red and blue locations shown in Figure 2.1. Traffic counts were conducted in 15-minute increments by FHWA vehicle classifications for the 7-day period from October 22, 2015 through October 29, 2015. A description of the count locations is provided in Table 2.1.
- An automatic license plate recognition (ALPR) survey was conducted at 12 of the 15 traffic count locations. ALPR survey locations are shown in red on Figure 2.1. This effort consisted of recording license plate images by video camera and matching the plates across the survey locations to assist in determining car and truck trip lengths on US 74 between Wingate and I485. These ALPR surveys were conducted from 5:30 AM to 7:00 PM on Thursday, October 22, 2015.
- INRIX travel times and speeds were obtained for roads in the study area.

The collected data are described in the following sections.

### 2.2 NCDOT Average Annual Daily Traffic Volumes

In addition to the traffic counts conducted as part of this study, CDM Smith collected and reviewed traffic counts provided by NCDOT. Figure 2.2 shows the 2014 average annual daily traffic volumes at NCDOT count locations throughout the study region. As shown, the coverage area is quite extensive and includes multiple count locations along key competing and complementary routes to the proposed Monroe Expressway.


TRAFFIC COUNT AND AUTOMATIC LICENSE PLATE RECOGNITION SURVEY LOCATIONS

Table 2.1
Locations for Automatic License Plate Recognition and Vehicle Classification Counts

| Station ID (1) | Count Location | Data Collected Between These Crossroads |  |
| :---: | :---: | :---: | :---: |
|  |  | Crossroad 1 | Crossroad 2 |
| 1 | US 74 | Independence Commerce Drive | Stallings Road |
| 2 | US 74 | Indian Trail Fairview Road | Faith Church Road |
| 3 | US 74 | Chamber Drive | Breckenridge Center Drive |
| 4 | US 74 | Roland Drive | Secrest Short Cut Road |
| 5 | US 74 | Miller Street | NC 200/Morgan Mill Road |
| 6 | US 74 | S Bivens Road | Edgewood Drive |
| 7 | US 74 | Forest Hills Road | N. Austin Street |
| 8 | Old Charlotte Highway | I-485 | Morningwood Drive |
| 9 | North Charlotte Avenue | Dickerson Boulevard | Concord Ave |
| 10 | Idlewild Road | I-485 | Stevens Mill Road |
| 11 | Secrest Short Cut Road | Unionville Indian Trail Road W | N Rocky River Road |
| 12 | Secrest Short Cut Road | Kim Court | Euclid Street |
| 13 | NC 84 | Willoughby Road | Rocky River Road |
| 14 | NC 200 | Creekridge Drive | N. Sutherland Avenue |
| 15 | NC 218 | Price Tucker Road | US 601 |
| 1) Seven-day vehicle classification counts were conducted at all locations. ALPR surveys were conducted at Stations 1 through 13. |  |  |  |

AADT volumes on US 74 were about 15,000 just east of the eastern Project terminus, in the vicinity of Marshville. Volumes nearly doubled on US 74 just west of the eastern Project terminus, to about 28,000. Daily volumes then increase considerably (to between 48,000 and 56,000 ) in the vicinity of Monroe. Peak volumes were recorded just east of the interchange with I-485, where daily trips reach 60,000. Daily volumes along the secondary parallel route to the Project, Old Charlotte Highway, ranged from about 14,000 in the middle of the corridor, to a high of 26,000 just east of I-485. Idlewild Road/Secrest Short Cut Road, which largely follows much of the western alignment of the Monroe Expressway, had average daily traffic volumes that ranged from 9,800 to 20,000.

### 2.3 Summary of 7-Day Continuous Traffic Count Program

This section shares key information collected regarding daily and hourly traffic variations, as well as the mix of vehicles along key existing roads in the Project corridor.

Figure 2.3 presents both the 2015 average weekday and average weekend day traffic volumes and the Station ID at each of the 15 data collection points. At all locations, average weekday volumes were higher than average weekend day volumes. Along US 74 west of Monroe, average weekday volumes at count stations 1 through 4 were between 10.4 percent and 12.6 percent higher than weekend day volumes. Weekday volumes were relatively higher compared to weekend day volumes on US 74 at count stations 5 through 7 (in Monroe and between Monroe and Marshville). At these three locations, weekday volumes were 17.1 percent to 19.3 percent greater than weekend day volumes.


Source: NCDOT AADT Maps


Source: Counts conducted by The Traffic Group from October 22 through 29, 2015.
2015 AVERAGE ANNUAL WEEKDAY AND WEEKEND DAY TRAFFIC VOLUMES

These relatively higher weekday volumes would suggest a larger weekday commuter component compared to the other locations.

Most of the other count stations in and around the Monroe area exhibited a relatively high weekday traffic component, ranging from 6.3 percent to 43.5 percent higher than weekend day volumes. Again, this would suggest a high commuter base of traffic on these roads.

Table 2.2 provides a detailed review of the distribution of average weekday traffic volumes by time period for each of the 15 count locations. The time periods used in Table 2.2 are the same four periods used in the Metrolina Regional Model. As shown, the distribution of traffic by time period was fairly consistent across all 15 count stations. The AM Peak period ( 3 hours) consisted of 15 to 21 percent of total weekday traffic. The Midday ( 6 hours) consisted of about 31 to 40 percent of total weekday traffic. The PM Peak period (3 hours) consisted of about 20 to 27 percent of total weekday traffic, and the Overnight period (12 hours) consisted of about 21 to 27 percent of total weekday traffic.

Table 2.2
Distribution of 2015 Average Annual Weekday Traffic by Time Period

| Station | Route | Percent Distribution of Weekday Traffic |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM Peak | Midday | PM Peak | Overnight |  |
| ID |  | 6:30-9:30 AM | 9:30 AM-3:30 PM | 3:30-6:30 PM | 6:30 PM-6:30 AM | Total Day |
| 1 | US 74 | 19.7 \% | 34.5 \% | 20.2 \% | 25.6 \% | 100.0 \% |
| 2 | US 74 | 18.2 | 35.4 | 20.4 | 26.0 | 100.0 |
| 3 | US 74 | 18.3 | 34.7 | 20.5 | 26.6 | 100.0 |
| 4 | US 74 | 15.7 | 37.8 | 21.2 | 25.2 | 100.0 |
| 5 | US 74 | 15.4 | 37.3 | 21.5 | 25.7 | 100.0 |
| 6 | US 74 | 17.2 | 34.7 | 22.2 | 25.9 | 100.0 |
| 7 | US 74 | 17.5 | 34.7 | 22.0 | 25.8 | 100.0 |
| US 74 - | verage | 17.5 | 35.7 | 21.0 | 25.8 | 100.0 |
| 8 | Old Charlotte Hwy | 19.2 | 35.9 | 22.4 | 22.5 | 100.0 |
| 9 | North Charlotte Avenue | 15.7 | 39.8 | 23.3 | 21.2 | 100.0 |
| Old Cha | otte/N. Charlotte -Average | 18.1 | 37.1 | 22.7 | 22.1 | 100.0 |
| 10 | Idlewild Road | 19.2 | 32.4 | 24.5 | 23.9 | 100.0 |
| 11 | Secrest Short Cut Rd | 18.8 | 32.2 | 26.0 | 23.0 | 100.0 |
| 12 | Secrest Short Cut Rd | 18.3 | 32.7 | 26.5 | 22.5 | 100.0 |
| Idlewild | Secrest Road - Average | 18.9 | 32.4 | 25.3 | 23.4 | 100.0 |
| 13 | NC 84 | 21.4 | 31.4 | 25.6 | 21.6 | 100.0 |
| 14 | NC 200 | 19.6 | 32.8 | 23.9 | 23.7 | 100.0 |
| 15 | NC 218 | 18.7 | 33.6 | 25.6 | 22.1 | 100.0 |

Source: Based on traffic counts conducted by The Traffic Group from October 22 through 29, 2015.

Daily traffic variations are shown in Table 2.3 for US 74 count stations. The table shows average daily traffic variations by day of week and by two-axle, three-or-more axle, and total vehicles.

Along US 74 (Table 2.3) Monday through Thursday total volumes tended to be relatively similar (all near an index of 1.0). In all cases, Sunday represented the lowest travel day of the week, with indices between 80 percent and 85 percent of the average day. Friday, on the other hand, was the highest travel day at all stations, with total volume indices ranging 7 to 19 percent greater than average daily volumes.

Table 2.3
2015 Daily Traffic Variations On US 74

| Day of Week | Station 1 |  | Station 2 |  | Station 3 |  | Station 4 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Traffic Volume | Index | Traffic Volume | Index | Traffic Volume | Index | Traffic Volume | Index |
| Two-Axle Vehicles |  |  |  |  |  |  |  |  |
| Sunday | 46,664 | 0.85 | 39,007 | 0.88 | 33,734 | 0.90 | 36,672 | 0.87 |
| Monday | 56,260 | 1.03 | 45,722 | 1.03 | 38,356 | 1.02 | 42,474 | 1.00 |
| Tuesday | 52,629 | 0.96 | 42,563 | 0.96 | 36,621 | 0.97 | 40,678 | 0.96 |
| Wednesday | 55,489 | 1.01 | 44,098 | 0.99 | 36,919 | 0.98 | 40,765 | 0.96 |
| Thursday | 57,190 | 1.04 | 45,153 | 1.02 | 37,077 | 0.99 | 43,205 | 1.02 |
| Friday | 58,090 | 1.06 | 48,472 | 1.09 | 40,944 | 1.09 | 47,431 | 1.12 |
| Saturday | 57,563 | 1.05 | 46,264 | 1.04 | 39,514 | 1.05 | 45,386 | 1.07 |
| Average Day | 54,841 | 1.00 | 44,468 | 1.00 | 37,595 | 1.00 | 42,373 | 1.00 |
| Average Weekday | 55,932 | 1.02 | 45,202 | 1.02 | 37,983 | 1.01 | 42,911 | 1.01 |
| Average Weekend Day | 52,114 | 0.95 | 42,636 | 0.96 | 36,624 | 0.97 | 41,029 | 0.97 |


|  | Three-or-More Axle Vehicles |  |  |  |  |  |  |  |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Sunday | 1,535 | 0.37 | 1,487 | 0.38 | 1,236 | 0.36 | 1,175 | 0.36 |
| Monday | 5,140 | 1.25 | 4,828 | 1.24 | 4,229 | 1.22 | 3,836 | 1.17 |
| Tuesday | 5,062 | 1.23 | 4,721 | 1.21 | 4,204 | 1.21 | 3,924 | 1.20 |
| Wednesday | 5,030 | 1.23 | 4,687 | 1.20 | 4,219 | 1.22 | 4,012 | 1.22 |
| Thursday | 4,637 | 1.13 | 4,685 | 1.20 | 4,378 | 1.26 | 4,301 | 1.31 |
| Friday | 4,969 | 1.21 | 4,694 | 1.21 | 4,077 | 1.18 | 3,881 | 1.18 |
| Saturday | 2,342 | 0.57 | 2,161 | 0.55 | 1,909 | 0.55 | 1,801 | 0.55 |
|  |  |  |  |  |  |  |  |  |
| Average Day | 4,102 | 1.00 | 3,895 | 1.00 | 3,465 | 1.00 | 3,276 | 1.00 |
| Average Weekday | 4,968 | 1.21 | 4,723 | 1.21 | 4,221 | 1.22 | 3,991 | 1.22 |
| Average Weekend Day | 1,939 | 0.47 | 1,824 | 0.47 | 1,573 | 0.45 | 1,488 | 0.45 |

Total Vehicles

| Sunday | 48,199 | 0.82 | 40,494 | 0.84 | 34,970 | 0.85 | 37,847 | 0.83 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Monday | 61,400 | 1.04 | 50,550 | 1.05 | 42,585 | 1.04 | 46,310 | 1.01 |
| Tuesday | 57,691 | 0.98 | 47,284 | 0.98 | 40,825 | 0.99 | 44,602 | 0.98 |
| Wednesday | 60,519 | 1.03 | 48,785 | 1.01 | 41,138 | 1.00 | 44,777 | 0.98 |
| Thursday | 61,827 | 1.05 | 49,838 | 1.03 | 41,455 | 1.01 | 47,506 | 1.04 |
| Friday | 63,059 | 1.07 | 53,166 | 1.10 | 45,021 | 1.10 | 51,312 | 1.12 |
| Saturday | 59,905 | 1.02 | 48,425 | 1.00 | 41,423 | 1.01 | 47,187 | 1.03 |
|  |  |  |  |  |  |  |  |  |
| Average Day | 58,943 | 1.00 | 48,363 | 1.00 | 41,060 | 1.00 | 45,649 | 1.00 |
| Average Weekday | 60,899 | 1.03 | 49,925 | 1.03 | 42,205 | 1.03 | 46,901 | 1.03 |
| Average Weekend Day | 54,052 | 0.92 | 44,460 | 0.92 | 38,197 | 0.93 | 42,517 | 0.93 |

Source: Based on traffic counts conducted by The Traffic Group from October 22 through 29, 2015.

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Table 2.3 (Continued) 2015 Daily Traffic Variations On US 74

| Day of Week | Station 5 |  | Station 6 |  | Station 7 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Traffic Volume | Index | Traffic Volume | Index | Traffic Volume | Index |
| Two-Axle Vehicles |  |  |  |  |  |  |
| Sunday | 38,878 | 0.82 | 22,992 | 0.84 | 16,540 | 0.90 |
| Monday | 48,178 | 1.02 | 27,343 | 1.00 | 18,531 | 1.01 |
| Tuesday | 44,656 | 0.95 | 25,456 | 0.93 | 17,090 | 0.93 |
| Wednesday | 45,198 | 0.96 | 27,125 | 0.99 | 17,468 | 0.95 |
| Thursday | 52,710 | 1.12 | 28,287 | 1.04 | 19,092 | 1.04 |
| Friday | 53,899 | 1.14 | 32,033 | 1.17 | 21,899 | 1.19 |
| Saturday | 46,675 | 0.99 | 27,926 | 1.02 | 17,921 | 0.98 |
| Average Day | 47,171 | 1.00 | 27,309 | 1.00 | 18,363 | 1.00 |
| Average Weekday | 48,928 | 1.04 | 28,049 | 1.03 | 18,816 | 1.02 |
| Average Weekend Day | 42,777 | 0.91 | 25,459 | 0.93 | 17,231 | 0.94 |


|  | Three-or-More Axle Vehicles |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Sunday | 1,043 | 0.38 | 684 | 0.31 | 694 | 0.31 |
| Monday | 3,292 | 1.19 | 2,676 | 1.20 | 2,657 | 1.17 |
| Tuesday | 2,816 | 1.02 | 2,687 | 1.20 | 2,878 | 1.27 |
| Wednesday | 2,877 | 1.04 | 2,875 | 1.29 | 2,888 | 1.28 |
| Thursday | 4,310 | 1.56 | 3,002 | 1.34 | 3,040 | 1.34 |
| Friday | 3,566 | 1.29 | 2,669 | 1.19 | 2,654 | 1.17 |
| Saturday | 1,387 | 0.50 | 1,057 | 0.47 | 1,041 | 0.46 |
|  |  |  |  |  |  |  |
| Average Day | 2,756 | 1.00 | 2,236 | 1.00 | 2,265 | 1.00 |
| Average Weekday | 3,372 | 1.22 | 2,782 | 1.24 | 2,823 | 1.25 |
| Average Weekend Day | 1,215 | 0.44 | 871 | 0.39 | 868 | 0.38 |

Total Vehicles

| Sunday | 39,921 | 0.80 | 23,676 | 0.80 | 17,234 | 0.84 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Monday | 51,470 | 1.03 | 30,019 | 1.02 | 21,188 | 1.03 |
| Tuesday | 47,472 | 0.95 | 28,143 | 0.95 | 19,968 | 0.97 |
| Wednesday | 48,075 | 0.96 | 30,000 | 1.02 | 20,356 | 0.99 |
| Thursday | 57,020 | 1.14 | 31,289 | 1.06 | 22,132 | 1.07 |
| Friday | 57,465 | 1.15 | 34,702 | 1.17 | 24,553 | 1.19 |
| Saturday | 48,062 | 0.96 | 28,983 | 0.98 | 18,962 | 0.92 |
|  |  |  |  |  |  |  |
| Average Day | 49,926 | 1.00 | 29,545 | 1.00 | 20,628 | 1.00 |
| Average Weekday | 52,300 | 1.05 | 30,831 | 1.04 | 21,639 | 1.05 |
| Average Weekend Day | 43,992 | 0.88 | 26,330 | 0.89 | 18,098 | 0.88 |

Source: Based on traffic counts conducted by The Traffic Group from October 22 through 29, 2015.

Table 2.4

## 2015 Daily Traffic Variations On Alternative Roads

Old Charlotte Hwy. and
North Charlotte Ave.

| Day of Week | North Charlotte Ave. |  |  |  | Idlewild Rd. and Secrest Short Cut Rd. |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Station 8 |  | Station 9 |  | Station 10 |  | Station 11 |  | Station 12 |  |
|  | Traffic <br> Volume | Index | Traffic <br> Volume | Index | Traffic Volume | Index | Traffic Volume | Index | Traffic Volume | Index |
| Two Axle Vehicles |  |  |  |  |  |  |  |  |  |  |
| Sunday | 18,948 | 0.75 | 7,151 | 0.65 | 18,223 | 0.84 | 7,988 | 0.80 | 7,572 | 0.80 |
| Monday | 26,386 | 1.05 | 11,547 | 1.05 | 21,574 | 1.00 | 10,113 | 1.01 | 9,606 | 1.01 |
| Tuesday | 25,407 | 1.01 | 11,307 | 1.03 | 20,594 | 0.95 | 9,533 | 0.95 | 9,168 | 0.97 |
| Wednesday | 26,220 | 1.04 | 11,546 | 1.05 | 21,448 | 0.99 | 10,014 | 1.00 | 9,477 | 1.00 |
| Thursday | 27,299 | 1.09 | 12,234 | 1.11 | 23,049 | 1.06 | 10,731 | 1.07 | 9,887 | 1.04 |
| Friday | 28,052 | 1.12 | 13,501 | 1.23 | 25,088 | 1.16 | 11,908 | 1.19 | 11,350 | 1.20 |
| Saturday | 23,547 | 0.94 | 9,767 | 0.89 | 21,797 | 1.01 | 9,643 | 0.97 | 9,282 | 0.98 |
| Average Day | 25,123 | 1.00 | 11,008 | 1.00 | 21,682 | 1.00 | 9,990 | 1.00 | 9,477 | 1.00 |
| Average Weekday | 26,673 | 1.06 | 12,027 | 1.09 | 22,351 | 1.03 | 10,460 | 1.05 | 9,898 | 1.04 |
| Average Weekend Day | 21,248 | 0.85 | 8,459 | 0.77 | 20,010 | 0.92 | 8,816 | 0.88 | 8,427 | 0.89 |

Three-or-More Axle Vehicles

| Sunday | 72 | 0.14 | 23 | 0.18 | 76 | 0.27 | 38 | 0.17 | 35 | 0.20 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Monday | 796 | 1.53 | 154 | 1.20 | 372 | 1.32 | 322 | 1.45 | 295 | 1.68 |
| Tuesday | 554 | 1.07 | 135 | 1.05 | 288 | 1.02 | 219 | 0.99 | 174 | 0.99 |
| Wednesday | 508 | 0.98 | 151 | 1.18 | 223 | 0.79 | 227 | 1.02 | 165 | 0.94 |
| Thursday | 715 | 1.38 | 181 | 1.41 | 380 | 1.35 | 315 | 1.42 | 244 | 1.39 |
| Friday | 755 | 1.45 | 191 | 1.49 | 457 | 1.62 | 302 | 1.36 | 213 | 1.21 |
| Saturday | 242 | 0.47 | 64 | 0.50 | 181 | 0.64 | 130 | 0.59 | 104 | 0.59 |
|  |  |  |  |  |  |  |  |  |  |  |
| Average Day | 520 | 1.00 | 128 | 1.00 | 282 | 1.00 | 222 | 1.00 | 176 | 1.00 |
| Average Weekday | 666 | 1.28 | 162 | 1.27 | 344 | 1.22 | 277 | 1.25 | 218 | 1.24 |
| Average Weekend Day | 157 | 0.30 | 44 | 0.34 | 129 | 0.46 | 84 | 0.38 | 70 | 0.40 |

Total Vehicles

| Sunday | 19,020 | 0.74 | 7,174 | 0.64 | 18,299 | 0.83 | 8,026 | 0.79 | 7,607 | 0.79 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Monday | 27,182 | 1.06 | 11,701 | 1.05 | 21,946 | 1.00 | 10,435 | 1.02 | 9,901 | 1.03 |
| Tuesday | 25,961 | 1.01 | 11,442 | 1.03 | 20,882 | 0.95 | 9,752 | 0.95 | 9,342 | 0.97 |
| Wednesday | 26,728 | 1.04 | 11,697 | 1.05 | 21,671 | 0.99 | 10,241 | 1.00 | 9,642 | 1.00 |
| Thursday | 28,014 | 1.09 | 12,415 | 1.11 | 23,429 | 1.07 | 11,046 | 1.08 | 10,131 | 1.05 |
| Friday | 28,807 | 1.12 | 13,692 | 1.23 | 25,545 | 1.16 | 12,210 | 1.20 | 11,563 | 1.20 |
| Saturday | 23,789 | 0.93 | 9,831 | 0.88 | 21,978 | 1.00 | 9,773 | 0.96 | 9,386 | 0.97 |
|  |  |  |  |  |  |  |  |  |  |  |
| Average Day | 25,643 | 1.00 | 11,136 | 1.00 | 21,964 | 1.00 | 10,212 | 1.00 | 9,653 | 1.00 |
| Average Weekday | 27,338 | 1.07 | 12,189 | 1.09 | 22,695 | 1.03 | 10,737 | 1.05 | 10,116 | 1.05 |
| Average Weekend Day | 21,405 | 0.83 | 8,503 | 0.76 | 20,139 | 0.92 | 8,900 | 0.87 | 8,497 | 0.88 |

Source: Based on traffic counts conducted by The Traffic Group from October 22 through 29, 2015.

Passenger car traffic volumes on US 74 were quite consistent between average weekdays and average weekend days. Average weekday passenger car volumes ranged from 1 to 4 percent above the average daily volume, and average weekend volumes ranged from 3 to 9 percent below the average daily volume. As would be expected, commercial-vehicle travel was highly concentrated on weekdays. Commercial vehicle traffic on US 74 was 21 to 25 percent higher on a weekday compared to an average day. Weekend commercial vehicle volumes on US 74 were less than half that of average daily commercial traffic.

Daily traffic variations for two roads parallel to US 74 are presented in Table 2.4. These two roads, Old Charlotte Highway/North Charlotte Avenue and Idlewild Road/Secrest Short Cut Road share similar daily variations in traffic volumes. Passenger car volumes were consistently lowest on Sundays and highest on Fridays. Average weekday passenger car volumes ranged from 6 to 9 percent above the average day on Old Charlotte Highway/North Charlotte Avenue, and from 3 to 5 percent above the average day on Idlewild Road/Secrest Short Cut Road. Commercial vehicle traffic was highly concentrated on weekdays. Weekend commercial vehicle volumes were 30 to 46 percent of the average daily commercial volume, while average weekday volumes range from 22 to 28 percent above the average day.

Weekday hourly traffic variations, by direction, are shown graphically for four count stations along US 74 in Figure 2.4. Stations 1 and 3 (both west of Monroe) exhibit clear morning and evening peaking characteristics. Stations 5 and 6 (in Monroe and east of Monroe) had a slight morning peak and a gradual building of traffic throughout the day, culminating in an evening peak. What is most noticeable is the similarity in traffic volumes in each hour for between the two travel directions. There are some divergences, but they are relatively small.

Figure 2.5 depicts weekday hourly variations for count stations along Old Charlotte Highway (Station 8) and North Charlotte Avenue (Station 9), and Idlewild Road (Station 10) and Secrest Short Cut Road (Station 12). Count Stations 8 and 10 are both at the far western end of the Project corridor, near I485. Both exhibit very similar characteristics in that there is a pronounced westbound morning peak and a pronounced eastbound evening peak. At Station 12 on Secrest Short Cut Road, which is very close to downtown Monroe, the peaking directionality is reversed, with an eastbound morning peak and a westbound evening peak. At Station 9, on North Charlotte Highway in downtown Monroe, the hourly traffic profile is flatter, with a small peak in the morning hours and subsequently increasing volumes through the midday and into a small evening peak. Based on the traffic counts, the westbound traffic volumes were slightly higher for most of the day, including both the morning and evening peaks. smith






Table 2.5 provides information on the typical weekday vehicle-class composition at count stations on US 74, Old Charlotte Highway/North Charlotte Avenue, Idlewild Road/Secrest Short Cut Road, and NC 84, NC 200, and NC 218. Three vehicle categories (which align with the assumed toll class schedules) are provided; consisting of two axle, three axle, and four-or-more axle vehicles. Two axle vehicles comprised the vast majority of traffic at all count stations; ranging between 87.0 percent at Station 2 to 98.7 percent at Station 9.

Table 2.5
2015 Traffic Composition by Vehicle Class - Week Day


Source: Based on traffic counts conducted by The Traffic Group from October 22 through 29, 2015.

Three axle vehicles were the smallest component of traffic at all locations; ranging from 0.5 percent at Station 11 to 2.2 percent at Station 14. Larger commercial vehicles (4-or-more axles) represented 5.0 to 11.0 percent of the traffic at US 74 count stations, and a much smaller proportion on Old Charlotte Highway/North Charlotte Avenue and Idlewild Road/Secrest Short Cut Road.

### 2.4 Travel Speed Summary

A key element of model validation, was verifying that the model output accurately reflected current travel speeds on competing and complementary routes to the Monroe Expressway by time period. Two sources of travel time information were utilized in the validation process. The primary data source was INRIX, while secondary travel time data was collected by CDM Smith staff in order to confirm the INRIX information. Summaries of data from both sources are provided in this section.

Figure 2.6 identifies all the roads for which INRIX data was available. One full year of weekday data (November 2014 through October 2015) was requested for the highlighted roads in this figure. All of the data was summarized by hour, direction, and roadway segment.

Table 2.6 shows the summarized average weekday INRIX travel speed data for US 74 between NC 205/Elm Street in Marshville and I-485 near Matthews. Establishing actual travel speeds, particularly on US 74 is important, as the majority of traffic on the Monroe Expressway would come from US 74. The Monroe Expressway in intended to provide reliable high-speed travel to longer distance trips as an alternative to US 74 which provides local access to area homes and businesses. Posted speed limits on US 74 between NC 205/Elm Street in Marshville and I-485 range from 35 to 55 mph .

The visual "heat map" in Table 2.6 depicts travel speed by direction and hour, for road segments provided by INRIX. Each box shows the average weekday travel speed for a particular hour. Variations in travel speed can occur within the hour. The boxes are color coded to represent average travel speeds in 10 mile-per-hour increments. In general, travel speeds are slower in the westbound direction compared to eastbound travel. In the westbound direction, travel speeds showed the most extended declines between East Franklin Street and NC 200/Morgan Mill Road beginning around 7:00 AM through 10:00 PM (hour 21). As shown, speeds in this section ranged from 18 to 29 mph . During the same time period, average travel speeds were slightly faster on the adjacent segment, from NC 200/Morgan Hill Road to US 601, when speeds averaged around 30 mph .

Operating speeds were a little faster in the eastbound direction. The segment between I-485 and Stallings Road is the only segment that experienced average speeds less than 20 mph during a typical weekday. This condition occurred between 4:00 PM and 6:00 PM. The East Franklin Street to US 601/Pageland Highway segment exhibited the longest time period of suboptimal travel speeds; operating at around 30 mph between 10:00 AM and 4:00 PM.

Table 2.6
US 74 Weekday Travel Speeds from INRIX－Average from November 2014 through October 2015

| Segment Name | Distance （mi） | Westbound Speed by Hour |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 0 | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 |
| NC－205／Elm St． | 8.54 | 55 | 55 | 56 | 56 | 56 | 56 | 55 | 55 | 55 | 55 | 55 | 55 | 55 | 56 | 55 | 55 | 56 | 56 | 56 | 56 | 55 | 55 | 56 | 56 |
| US－601／Pageland Hwy． | 0.12 | 48 | 48 | 48 | 48 | 48 | 48 | 47 | 46 | 46 | 46 | 47 | 46 | 46 | 47 | 46 | 46 | 45 | 46 | 46 | 47 | 47 | 47 | 48 | 48 |
| E．Franklin St． | 1.21 | 34 | 34 | 35 | 35 | 34 | 35 | 32 | 29 | 28 | 21 | 20 | 20 | 18 | 19 | 19 | 19 | 20 | 21 | 23 | 29 | 28 | 28 | 31 | 33 |
| NC－200／Morgan Mill Rd． | 1.02 | 38 | 38 | 38 | 39 | 38 | 38 | 34 | 30 | 30 | 30 | 30 | 29 | 28 | 29 | 29 | 29 | 28 | 28 | 30 | 32 | 33 | 33 | 35 | 36 |
| US－601（Concord Hwy）／NC－200 | 0.35 | 43 | 44 | 44 | 44 | 45 | 44 | 43 | 43 | 42 | 39 | 39 | 37 | 35 | 37 | 37 | 36 | 35 | 36 | 38 | 42 | 41 | 39 | 41 | 42 |
| US－601（Concord Hwy）／NC－200 | 1.58 | 46 | 46 | 46 | 46 | 47 | 47 | 47 | 47 | 47 | 46 | 45 | 45 | 43 | 44 | 44 | 45 | 45 | 44 | 46 | 46 | 45 | 44 | 45 | 46 |
| Roland Dr． | 6.86 | 40 | 41 | 41 | 41 | 41 | 42 | 40 | 38 | 36 | 35 | 33 | 30 | 27 | 28 | 30 | 31 | 32 | 32 | 32 | 34 | 35 | 33 | 37 | 39 |
| Indian Trail Fairview Rd． | 1.27 | 48 | 48 | 49 | 49 | 49 | 47 | 43 | 38 | 39 | 43 | 44 | 43 | 40 | 39 | 39 | 38 | 37 | 35 | 37 | 41 | 42 | 42 | 45 | 47 |
| Stallings Rd． | 0.75 | 49 | 49 | 50 | 50 | 50 | 49 | 41 | 24 | 23 | 37 | 45 | 45 | 43 | 43 | 44 | 42 | 40 | 33 | 34 | 38 | 42 | 43 | 46 | 48 |
| 1－485 | 0.76 | 51 | 51 | 52 | 52 | 52 | 52 | 48 | 44 | 43 | 47 | 49 | 49 | 48 | 48 | 48 | 48 | 48 | 45 | 45 | 43 | 46 | 47 | 48 | 50 |

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|  |  | US-601/Pageland Hwy. |  |  |  | US－601（Concord Hwy）／NC－200 |  |  |  | $\xrightarrow{\text {－1 }}$ |

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CDM Smith performed a limited number of travel time runs to validate the reasonableness of the INRIX speed data. Travel time runs were conducted on US 74 and Old Charlotte Highway/North Charlotte Avenue on November 12, 2015. A summary of the travel time runs is provided in Table 2.7.

The INRIX travel speeds exceeded the travel time study speeds by about 4 to 11 percent on US 74 and about 4 to 10 percent on Old Charlotte Highway/North Charlotte Avenue. It was concluded that the INRIX data is an acceptable indicator of current travel speeds for this study. The percent differences in travel speeds between the two sources were generally below 10 percent. The differences are likely due to the greater number of INRIX data points that represent all months out of a year, compared to the limited number of travel time studies conducted by CDM Smith.

Table 2.7
Validation of INRIX Travel Times and Speeds

| Direction | Period | Travel Time Studies (TTS) (1) |  |  | INRIX |  | Percent Difference INRIX to TTS |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Number of Runs | $\begin{gathered} \hline \text { Travel } \\ \text { Time (2) } \\ \hline \end{gathered}$ | Speed <br> (mph) | Travel Time (2) | Speed <br> (mph) | $\begin{gathered} \hline \text { Travel } \\ \text { Time (2) } \\ \hline \end{gathered}$ | Speed <br> (mph) |
| Westbound |  | US 74 (3) |  |  |  |  |  |  |
|  | AM | 3 | 0:32:44 | 35.0 | 0:29:52 | 38.4 | (8.8) | 9.6 |
|  | MD | 2 | 0:27:10 | 42.2 | 0:26:12 | 43.8 | (3.6) | 3.8 |
|  | PM | 3 | 0:32:57 | 34.8 | 0:30:47 | 37.2 | (6.6) | 7.0 |
| Eastbound | AM | 3 | 0:27:39 | 41.7 | 0:26:51 | 42.9 | (2.9) | 3.0 |
|  | MD | 2 | 0:30:41 | 37.6 | 0:28:29 | 40.4 | (7.1) | 7.7 |
|  | PM | 2 | 0:34:35 | 33.3 | 0:31:14 | 36.9 | (9.7) | 10.7 |
| Westbound | $\begin{aligned} & \mathrm{AM} \\ & \mathrm{MD} \end{aligned}$ | Old Charlotte Highway/North Charlotte Avenue (4) |  |  |  |  |  |  |
|  |  | 2 | 0:30:07 | 24.5 | 0:27:54 | 26.4 | (7.3) | 7.9 |
|  |  |  |  |  |  |  |  |  |
|  | PM | 2 | 0:25:20 | 29.1 | 0:24:21 | 30.3 | (3.9) | 4.1 |
| Eastbound | AM | 2 | 0:24:57 | 29.6 | 0:22:40 | 32.6 | (9.1) | 10.1 |
|  | MD |  |  |  |  |  |  |  |
|  | PM | 2 | 0:26:46 | 27.6 | 0:25:46 | 28.6 | (3.8) | 3.9 |
| 1) Travel time runs conducted by CDM Smith |  |  |  |  |  |  |  |  |
| 2) Travel time in minutes and seconds |  |  |  |  |  |  |  |  |
| 3) Travel time between I-485 and Forest Hills Road in Marshville, NC |  |  |  |  |  |  |  |  |
| 4) Travel time between I-485 and W. Cromwell Street in Monroe, NC |  |  |  |  |  |  |  |  |

### 2.5 License Plate Surveys

Automatic license plate recognition surveys were conducted at 12 locations within the study corridor from 5:30 AM to 7:00 PM on October 22, 2015. These locations are shown in Figure 2.1. The purpose of the study was to gather data on trip lengths, particularly on US 74, and compare the collected triplength data to estimated trip lengths in the Metrolina Model. This data is beneficial to calibrating the model because motorists currently on US 74 would be much more likely to use the Monroe Expressway for longer distance trips compared to very short trips.

License plate images were collected in the westbound direction only. License plate images were processed using optical character recognition software, followed by a manual image review to ensure accuracy. Vehicle classification for each record was assigned during the manual image review process and was limited to distinguishing between passenger cars (FHWA Classes 1-5) and commercial vehicles (FHWA Classes 6-13).

Table 2.8 presents the number of successfully captured license plate images at each location by time of day and vehicle class. Successfully captured license plate images are also presented as a percent of passing traffic during the survey period. Several instances of capture rates in excess of 100 percent are shown for commercial vehicles. This is likely due to incorrect vehicle classification of a small proportion of commercial vehicles in the manual image review process. The key point is that the sample size is very large. For US 74 locations, the successful identification rate of passenger car plates ranged from 81 to 94 of passing traffic on a total day. The successful identification of commercial vehicle trips on US 74 ranged from 67 to 94 percent on a total day.

Captured license-plate images were matched across survey stations and checked for reasonableness based on logical movements and travel time to identify unique trips. The results of the ALPR survey for trips on US 74 are summarized in Table 2.9 for a weekday AM period, PM period and total day, for passenger cars and commercial vehicles. All movements reflect westbound trips along US 74. The station where a plate was first identified (the origin) is shown in the left column, while the last station where the same plate was identified (the destination) is shown across the top. The percent distribution of trips from each origin station are unique, thus, the sum of each row adds up to 100 percent. Each row describes movements that are identified as a percentage of the trips that started at a station and traveled far enough to be captured at a downstream station.

For example, for passenger cars in the AM time period, 4 percent of trips with an origin at Station 7 went as far as Station 3 (but did not reach Station 2), another 4 percent had a destination that went as far as Station 2 (but did not reach Station 1), and 17 percent had a destination that went as far as Station 1, or beyond. Any trip that was captured at Station 7, but did not also pass through Stations 6, 5, 4, 3, 2, or 1, would not be represented in this table.

Commercial trucks exhibited a higher percentage of long distance trips than passenger cars. On a total day basis, 39 percent of trips first identified at Station 7 continued through Station 1, compared to 19 percent for passenger cars, and 58 percent of commercial vehicles that entered at Station 5 continued through Station 1 compared to 34 percent of passenger cars.

It is important to recognize that these are not complete trip tables, as US 74 is not a limited access roadway. The number of access points are numerous, and this data set represents snapshots of specific locations, chosen to fall between major intersecting roads with US 74. However, the data was useful for comparing against trip-distance distributions in the Metrolina Model. Adjustments were made to the MRM's trip table to adjust for the observed trip distances.
Table 2.8

| Survey <br> Station | Total Day: (5:30 AM - 7:00 PM) |  |  |  |  |  | AM Period (7:00 AM - 10:00 AM) |  |  |  |  |  | PM Period (3:00 PM - 7:00 PM) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Passenger Cars |  |  | Commercial Vehicles |  |  | Passenger Cars |  |  | Commercial Vehicles |  |  | Passenger Cars |  |  | Commercial Vehicles |  |  |
|  | Passing Traffic | Captured <br> License <br> Plates | Percent Capture Rate | Passing <br> Traffic | Captured <br> License Plates | Percent Capture Rate | Passing <br> Traffic | Captured <br> License Plates | Percent <br> Capture Rate | Passing <br> Traffic | Captured <br> License <br> Plates | Percent Capture Rate | Passing <br> Traffic | Captured <br> License <br> Plates | Percent <br> Capture Rate | Passing <br> Traffic | Captured <br> License <br> Plates | Percent <br> Capture Rate |
| 1 | 25,293 | 23,792 | 94\% | 1,661 | 1,469 | 88\% | 6,516 | 5,176 | 79\% | 306 | 327 | 107\% | 7,790 | 7,554 | 97\% | 430 | 399 | 93\% |
| 2 | 18,335 | 16,035 | 87\% | 1,536 | 1,380 | 90\% | 4,100 | 3,063 | 75\% | 278 | 295 | 106\% | 5,966 | 5,600 | 94\% | 400 | 369 | 92\% |
| 3 | 15,411 | 14,251 | 92\% | 1,333 | 1,081 | 81\% | 3,536 | 3,195 | 90\% | 273 | 192 | 70\% | 4,979 | 4,620 | 93\% | 346 | 310 | 90\% |
| 4 | 18,357 | 17,009 | 93\% | 1,342 | 1,051 | 78\% | 3,390 | 2,891 | 85\% | 276 | 216 | 78\% | 6,074 | 5,739 | 94\% | 319 | 273 | 86\% |
| 5 | 22,216 | 18,098 | 81\% | 1,238 | 1,162 | 94\% | 4,074 | 3,082 | 76\% | 224 | 232 | 104\% | 7,352 | 6,373 | 87\% | 320 | 282 | 88\% |
| 6 | 11,773 | 9,984 | 85\% | 942 | 682 | 72\% | 2,313 | 2,003 | 87\% | 164 | 125 | 76\% | 4,108 | 3,286 | 80\% | 289 | 168 | 58\% |
| 7 | 8,084 | 7,389 | 91\% | 965 | 651 | 67\% | 1,668 | 1,545 | 93\% | 175 | 142 | 81\% | 2,814 | 2,466 | 88\% | 277 | 155 | 56\% |
| 8 | 11,066 | 9,017 | 81\% | 273 | 204 | 75\% | 3,115 | 2,165 | 70\% | 48 | 36 | 75\% | 3,553 | 1,932 | 54\% | 66 | 45 | 68\% |
| 9 | 5,819 | 4,899 | 84\% | 54 | 72 | 133\% | 1,097 | 924 | 84\% | 10 | 11 | 110\% | 1,957 | 1,794 | 92\% | 18 | 21 | 117\% |
| 10 | 10,274 | 8,976 | 87\% | 138 | 98 | 71\% | 2,778 | 2,187 | 79\% | 37 | 13 | 35\% | 2,976 | 2,845 | 96\% | 31 | 22 | 71\% |
| 11 | 4,582 | 3,983 | 87\% | 115 | 71 | 62\% | 903 | 733 | 81\% | 29 | 17 | 59\% | 1,692 | 1,496 | 88\% | 29 | 13 | 45\% |
| 12 | 4,364 | 3,249 | 74\% | 87 | 40 | 46\% | 1,054 | 472 | 45\% | 11 | 3 | 27\% | 1,545 | 1,476 | 96\% | 25 | 14 | 56\% |

Table 2.9
Results of Vehicle License Plate Recognition Survey Based on Westbound Trips on Thursday, October 22, 2015

| Passenger Cars |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AM Time Period (7:00 AM - 10:00 AM) |  |  |  |  |  |  |  |  |
| Origin Station |  | Destination Station |  |  |  |  |  |  |
|  |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| US 74 | 1 | Cllllow |  |  |  |  |  |  |
|  | 2 | 100\% | Cllll |  |  |  |  |  |
|  | 3 | 60\% | 40\% | (1) 1 Il |  |  |  |  |
|  | 4 | 44\% | 24\% | 32\% | (lllla |  |  |  |
|  | 5 | 32\% | 16\% | 20\% | 32\% | lla |  |  |
|  | 6 | 20\% | 10\% | 11\% | 18\% | 42\% | MIMMIM |  |
|  | 7 | 17\% | 4\% | 4\% | 9\% | 17\% | 50\% | - $\mathrm{ll} / \mathrm{ll}$ |


| Commercial Vehicles |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AM Time Period (7:00 AM - 10:00 AM) |  |  |  |  |  |  |  |  |
| First Identified At This Station |  | Destination Station |  |  |  |  |  |  |
|  |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| US 74 | 1 | Ololom |  |  |  |  |  |  |
|  | 2 | 100\% | VITIIT |  |  |  |  |  |
|  | 3 | 89\% | 11\% | Cll |  |  |  |  |
|  | 4 | 52\% | 22\% | 26\% | - |  |  |  |
|  | 5 | 71\% | 8\% | 14\% | 7\% | ITM |  |  |
|  | 6 | 42\% | 16\% | 0\% | 11\% | 32\% | (llm) |  |
|  | 7 | 46\% | 4\% | 10\% | 7\% | 8\% | 24\% | (llom |


| PM Time Period (3:00 PM - 7:00 PM) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin Station |  | Destination Station |  |  |  |  |  |  |
|  |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| US 74 | 1 | Olllla |  |  |  |  |  |  |
|  | 2 | 100\% | Cll 1 |  |  |  |  |  |
|  | 3 | 60\% | 40\% | Clllla |  |  |  |  |
|  | 4 | 48\% | 26\% | 26\% | (lu) |  |  |  |
|  | 5 | 37\% | 14\% | 10\% | 40\% | 业 |  |  |
|  | 6 | 20\% | 8\% | 6\% | 19\% | 47\% | VIllla |  |
|  | 7 | 23\% | 5\% | 3\% | 7\% | 21\% | 42\% | - 1 Il |


| First Identified At This Station |  | Destination Station |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| US 74 | 1 | (100\% |  |  |  |  |  |  |
|  | 2 | 100\% | Cll |  |  |  |  |  |
|  | 3 | 69\% | 31\% | Illll |  |  |  |  |
|  | 4 | 61\% | 22\% | 16\% | IVI |  |  |  |
|  | 5 | 62\% | 11\% | 9\% | 19\% | \% |  |  |
|  | 6 | 27\% | 0\% | 15\% | 27\% | 31\% | (1)lla |  |
|  | 7 | 43\% | 6\% | 7\% | 6\% | 11\% | 26\% | (lllllu |


| Total Day Time Period (5:30 AM - 7:00 PM) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Origin Station |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| US 74 | 1 |  |  |  |  |  |  |  |
|  | 2 | 100\% | Illl |  |  |  |  |  |
|  | 3 | 62\% | 38\% | (lllll |  |  |  |  |
|  | 4 | 47\% | 26\% | 27\% | Olo |  |  |  |
|  | 5 | 34\% | 13\% | 13\% | 40\% |  |  |  |
|  | 6 | 20\% | 8\% | 7\% | 21\% | 45\% | Cllll |  |
|  | 7 | 19\% | 4\% | 3\% | 11\% | 20\% | 44\% | -1] $l$ ll |


| Total Day Time Period (5:30 AM - 7:00 PM) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| First Identified At This Station |  | Destination Station |  |  |  |  |  |  |
|  |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| US 74 | 1 | IVI I |  |  |  |  |  |  |
|  | 2 | 100\% |  |  |  |  |  |  |
|  | 3 | 77\% | 23\% | Ol 1 l |  |  |  |  |
|  | 4 | 58\% | 21\% | 21\% | - |  |  |  |
|  | 5 | 58\% | 11\% | 11\% | 19\% | S0\% |  |  |
|  | 6 | 26\% | 11\% | 6\% | 27\% | 30\% | -1] |  |
|  | 7 | 39\% | 6\% | 8\% | 14\% | 11\% | 23\% |  |

Source: ALPR conducted by The Traffic group on October 22, 2015.

## Chapter 3

## Network Refinement

CDM Smith obtained the latest regional transportation demand model from the Charlotte Department of Transportation (CDOT). This section describes the model, the review of the model network, and subsequent refinements to the network in the study area.

### 3.1 The Metrolina Regional Model

The Metrolina Regional Model is the primary modeling tool for evaluating existing and future travel demand in the greater Charlotte area. The current model is in TransCAD and is designated the Metrolina Regional Travel Demand Model 2015 Version 1.1 (MRM15v1.1 or MRM). It covers a twelve county, bi-state region. In North Carolina, all of Cabarrus, Cleveland, Gaston, Lincoln, Mecklenburg, Rowan, Stanly and Union Counties are included, as well as the portion of Iredell County from the Mecklenburg County line northward to the Yadkin River, and the southeast corner of Catawba County. The model area also includes the entirety of York County, SC and the northern panhandle of Lancaster County, SC. The area encompassed by the MRM15v1.1 boundaries can be seen in Figure 3.1. The detailed study area is shaded in green. The proposed Monroe Expressway is shown in red.

The MRM is cooperatively developed and maintained by a team of modelers from the following agencies:

1. North Carolina Department of Transportation (NCDOT),
2. South Carolina Department of Transportation (SCDOT),
3. Charlotte Regional Transportation Planning Organization (CRTPO),
4. Cabarrus-Rowan Metropolitan Planning Organization (CRMPO),
5. Gaston - Cleveland - Lincoln Metropolitan Planning Organization (GCLMPO),
6. Rock Hill - Fort Mill Area Transportation Study (RFATS),
7. Charlotte Department of Transportation (CDOT) and,
8. Rocky River Rural Planning Organization (RRRPO).

The official base year of the MRM15v1.1 is 2010, which incorporates 2010 US Census data. The model forecast years include 2015, 2025, 2030 and 2040. Each model year contains planned roadway and transit improvements based on the financially constrained metropolitan transportation plans for each of the respective MPOs. Projects in the non-MPO (RPO) areas are reflected in the networks only if they were included in the respective NC and SC Transportation Improvement Plan (TIP) documents.

The MRM is a weekday model, consisting of 3,439 TAZs, 82 externals, and with the following defined time periods:

- AM Peak Period: 6:30 AM to 9:30 AM
- Midday: 9:30 AM to 3:30 PM
- PM Peak Period: 3:30 PM to 6:30 PM
- Overnight Period: 6:30 PM to 6:30 AM


METROLINA REGIONAL MODEL AREA MAP

In the fall of 2015, CDM Smith obtained a working version of the MRM15v1.1 and the model documentation. CDM Smith used the 2015 network as the base year, and 2025, 2030 and 2040 networks for future-year assignments. A review of the MRM networks was conducted to determine whether any refinements were needed to improve the modeling output for this study. The review included the network coding of the Monroe Expressway, the size of TAZs in the study area, the location of centroid connectors in the study area, and whether additional roads needed to be added to the networks.

### 3.2 Project Configuration

The most up-to-date, detailed Monroe Expressway configuration was used to code the project. Figure 3.2 presents a not-to-scale schematic that shows the location and configuration of the interchanges and the number of through travel lanes on the Expressway. The blue highlighted lanes represent the tolled Monroe Expressway and the green highlighted lanes represent the non-tolled, upgraded section of US 74. The distance in miles between interchanges on the tolled Monroe Expressway are shown in red. Four continuous, through travel lanes (two per direction) will be provided on the tolled Monroe Expressway through the forecast period, from 2019 through 2058.

Figure 3.3 is a schematic of the upgraded section of US 74, the frontage road system, and the partial interchange with the Monroe Expressway. There will not be any toll collection on the upgraded section of US 74 or the frontage roads, highlighted in green and orange, respectively. The upgraded section of US 74 will maintain four to six through travel lanes. The system of frontage roads and the upgraded section of US 74 will provide multiple opportunities for vehicles to move between US 74 and the tolled Monroe Expressway. The Monroe Expressway will have a posted speed limit of 65 mph .

The Monroe Expressway was already coded into the MRM network years 2025, 2030 and 2040. The project coding, including distances, access points, and number of lanes was compared to the Monroe Bypass Constructors RFC Roadway Plans for accuracy. No substantive changes to distances, access points, or number of lanes were required.

### 3.3 Traffic Analysis Zones, Centroid Connectors, and Added Roadways

The networks contain geographic areas called traffic analysis zones. Some TAZs in the study area were too large for the purposes of this study as they would not accurately differentiate how motorists would access the project. In order to more accurately reflect travel distances and times, CDM Smith disaggregated some TAZs in the vicinity of the proposed Monroe Expressway. Twenty TAZs were split into two, three or four new zones. The disaggregated zones are shown in Figure 3.4. Each original TAZ has one color, split by a dark black border.

After a review of the 2015 network and aerials available on the internet, it was determined that centroid connections for some TAZs needed to be modified in existing TAZs to more accurately reflect where traffic would access the roadway system. Centroid connectors represent where traffic loads onto the roadway network in each TAZ. Depending upon the TAZ, a centroid connector may have been re-located, deleted, or added. Modifications to centroid connectors were made to 19 existing TAZs and to the 20 disaggregated TAZs.


Not To Scale



NETWORK TAZ DISAGGREGATION

Several existing roads, or sections of roads, were added to networks in order to more accurately reflect likely travel patterns of motorists that would potentially access the Monroe Expressway. These roads included Mills Harris Road, Nash Road, McIntyre Road, Ellis Griffin Road, Camden Road, Price Dairy Road, and Oak Spring Road.

### 3.4 Review of 2015 Network to Actual Ground Conditions

A review of the 2015 network was made, comparing actual ground conditions from aerials available on internet mapping sites to the roads coded in the network. The review focused on validating distances, intersection access points and number of travel lanes. This review focused primarily in the vicinity of the study area. No substantive changes were required, with the exception of those changes described in Section 3.3.

## Chapter 4

## Independent Economic Review

Economic growth forecasts are one of the most critical elements of any traffic and revenue forecast, particularly for a new toll facility such as the proposed Monroe Expressway. Because of the inherent uncertainty in the economic forecasting process, this has also become an area of considerable review and scrutiny by rating agencies and others in the financial community. As such, CDM Smith engaged Dr. Stephen J. Appold, an economist with local expertise, to conduct an independent analysis of the economic growth forecasts assumed in the Metrolina Regional Travel Demand Model (MRM15v1.1), a key tool for evaluating future travel demand in the region.

The goal of this effort was to evaluate the reasonableness of regional and corridor growth rates contained in the MRM15v1.1 and to make adjustments where appropriate. In this process Dr. Appold identified and analyzed major employers, employment centers, housing developments, and commercial and retail developments in order to derive an understanding of the economic drivers of the region. He also conducted an analysis of state-wide, regional and corridor growth rates based on the latest available historic trends and forecasts.

Dr. Appold's review specifically addressed forecasts for population, numbers of households, and employment, all of which are directly related to the traffic forecasts that the MRM15v1.1 produces. Each of these parameters was addressed for forecast years officially supported by the MRM15v1.1 (2015, 2025, 2030 and 2040) and for years used for forecast interpolation purposes (2020 and 2035).

The following presents a summary of the existing socioeconomic projections contained in the MRM15v1.1 in comparison with forecasts developed by Dr. Appold. Forecasts are presented on a region-wide basis, with a focus on Mecklenburg and Union Counties, and subsequently with a focus on the study corridor itself. The adjusted data set was reviewed by CDM Smith and was used as input in the travel demand modeling conducted in support of this study. A detailed description of Dr. Appold's methodology and conclusions can be found in his February 2016 Evaluation of the Socio-economic Estimates Underlying the Study of the Feasibility of the Proposed Monroe Expressway.

### 4.1 Model Area Adjustments

### 4.1.1 Model Area Boundaries

The MRM15v1.1 includes 10 North Carolina counties: Cabarrus, Catawba, Cleveland, Gaston, Iredell, Lincoln, Mecklenburg, Rowan, Stanly, and Union; and two South Carolina counties: Lancaster and York. Of the 12 counties, Catawba, Iredell, and Lancaster are only partially covered in the MRM. The area encompassed by the MRM15v1.1 boundary can be seen in Figure 3.1. The detailed study area is shaded in green. Year 2010 is the official base year, supported by 2010 US Census data. Future year socio-economic and land-use assumptions in the MRM15v1.1 are forecasts, including year 2015.

### 4.1.2 Model Area Population

Table 4.1 presents MRM15v1.1 population projections by county for each model year. The upper portion of Table 4.1 shows the original data set. The lower portion shows the data set adjusted by Dr. Appold. Population growth rates for each 5-year interval are calculated using Average Annual Percent Change (AAPC).
Table 4.1
Model Area Population Forecasts by County - MRM15v1.1 and Adjusted Data

| County | MRM15v1.1 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AAPC |  | AAPC |  |  | AAPC | AAPC |  |  | AAPC |  | AAPC |  | AAPC |
|  | 2010 | 2010-15 | 2015 | 2015-20 | 2020 | 2020-25 | 2025 | 2025-30 | 2030 | 2030-35 | 2035 | 2035-40 | 2040 | 2010-40 |
| Cabarrus | 178,012 | 2.1\% | 197,837 | 3.3\% | 232,934 | 2.8\% | 267,877 | 3.5\% | 318,094 | 1.2\% | 337,916 | 1.1\% | 357,601 | 2.4\% |
| Catawba | 9,978 | 2.9\% | 11,517 | 2.0\% | 12,704 | 2.0\% | 14,036 | 2.0\% | 15,523 | 2.0\% | 17,147 | 2.0\% | 18,886 | 2.1\% |
| Cleveland | 98,072 | 0.2\% | 98,968 | 0.3\% | 100,367 | 0.9\% | 105,058 | 1.0\% | 110,361 | 1.4\% | 118,270 | 1.3\% | 126,103 | 0.8\% |
| Gaston | 206,086 | 0.8\% | 214,374 | 0.8\% | 222,605 | 0.7\% | 230,793 | 0.7\% | 238,816 | 0.7\% | 247,798 | 0.7\% | 256,689 | 0.7\% |
| Iredell | 145,359 | 1.4\% | 155,570 | 1.3\% | 165,746 | 1.6\% | 179,804 | 1.5\% | 193,845 | 1.5\% | 208,843 | 1.4\% | 223,790 | 1.4\% |
| Lincoln | 78,265 | 1.8\% | 85,587 | 1.6\% | 92,858 | 1.5\% | 100,170 | 1.4\% | 107,433 | 1.4\% | 115,056 | 1.3\% | 122,641 | 1.5\% |
| Mecklenburg | 919,628 | 2.0\% | 1,016,280 | 1.8\% | 1,112,324 | 1.6\% | 1,206,967 | 1.5\% | 1,301,009 | 1.4\% | 1,396,793 | 1.3\% | 1,491,996 | 1.6\% |
| Rowan | 138,428 | 0.5\% | 142,241 | 0.9\% | 148,962 | 0.9\% | 155,571 | 1.2\% | 165,088 | 0.5\% | 168,900 | 0.4\% | 172,602 | 0.7\% |
| Stanly | 60,585 | 0.5\% | 62,036 | 0.4\% | 63,392 | 1.0\% | 66,737 | 1.0\% | 70,000 | 1.2\% | 74,349 | 1.1\% | 78,602 | 0.9\% |
| Union | 201,288 | 2.6\% | 228,719 | 2.0\% | 252,424 | 1.7\% | 274,861 | 1.6\% | 297,021 | 1.4\% | 319,173 | 1.3\% | 341,132 | 1.8\% |
| Lancaster | 19,729 | 0.3\% | 20,032 | 0.9\% | 20,974 | 1.6\% | 22,718 | 2.8\% | 26,095 | 2.7\% | 29,845 | 2.6\% | 33,895 | 1.8\% |
| York | 226,073 | 1.9\% | 248,692 | 2.3\% | 278,117 | 2.1\% | 307,957 | 1.0\% | 322,899 | 1.1\% | 340,394 | 1.4\% | 365,266 | 1.6\% |
| Model Area Total | 2,281,503 | 1.7\% | 2,481,853 | 1.7\% | 2,703,407 | 1.6\% | 2,932,549 | 1.5\% | 3,166,184 | 1.3\% | 3,374,484 | 1.2\% | 3,589,203 | 1.5\% |
|  | Adjusted Data |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | AAPC |  | AAPC |  |  | AAPC | AAPC |  |  | AAPC | AAPC |  |  | AAPC |
| County | 2010 | 2010-15 | 2015 | 2015-20 | 2020 | 2020-25 | 2025 | 2025-30 | 2030 | 2030-35 | 2035 | 2035-40 | 2040 | 2010-40 |
| Cabarrus | 178,012 | 1.6\% | 192,856 | 3.0\% | 223,625 | 2.6\% | 254,149 | 3.2\% | 297,582 | 1.1\% | 314,610 | 1.1\% | 331,483 | 2.1\% |
| Catawba | 9,978 | 0.2\% | 10,097 | 1.2\% | 10,714 | 1.2\% | 11,387 | 1.2\% | 12,106 | 1.2\% | 12,866 | 1.2\% | 13,645 | 1.0\% |
| Cleveland | 98,072 | -0.1\% | 97,698 | 0.3\% | 99,059 | 0.9\% | 103,499 | 0.9\% | 108,498 | 1.3\% | 115,843 | 1.2\% | 123,167 | 0.8\% |
| Gaston | 206,086 | 0.5\% | 211,488 | 0.7\% | 218,619 | 0.7\% | 226,332 | 0.7\% | 233,885 | 0.7\% | 242,557 | 0.7\% | 251,139 | 0.7\% |
| Iredell | 145,359 | 1.1\% | 153,613 | 1.1\% | 161,900 | 1.6\% | 175,059 | 1.5\% | 188,213 | 1.2\% | 199,681 | 1.2\% | 212,169 | 1.3\% |
| Lincoln | 78,265 | 0.6\% | 80,545 | 1.5\% | 86,932 | 1.4\% | 92,989 | 1.3\% | 98,952 | 1.2\% | 105,120 | 1.1\% | 111,228 | 1.2\% |
| Mecklenburg | 919,628 | 2.1\% | 1,018,549 | 1.8\% | 1,115,877 | 1.5\% | 1,204,019 | 1.4\% | 1,291,649 | 1.4\% | 1,382,555 | 1.3\% | 1,473,062 | 1.6\% |
| Rowan | 138,428 | 0.0\% | 138,771 | 0.9\% | 145,250 | 0.8\% | 151,049 | 1.1\% | 159,380 | 0.4\% | 162,684 | 0.4\% | 165,913 | 0.6\% |
| Stanly | 60,585 | 0.2\% | 61,053 | 0.3\% | 62,054 | 1.0\% | 65,345 | 0.9\% | 68,408 | 1.2\% | 72,463 | 1.1\% | 76,421 | 0.8\% |
| Union | 201,288 | 1.8\% | 220,194 | 1.7\% | 239,703 | 1.7\% | 260,900 | 1.6\% | 281,836 | 1.4\% | 302,775 | 1.3\% | 323,524 | 1.6\% |
| Lancaster | 19,729 | 0.3\% | 20,032 | 0.9\% | 20,974 | 1.0\% | 22,005 | 1.7\% | 23,966 | 1.7\% | 26,027 | 1.6\% | 28,133 | 1.2\% |
| York | 226,073 | 2.0\% | 249,185 | 2.3\% | 279,025 | 2.0\% | 308,773 | 0.9\% | 323,633 | 1.1\% | 341,576 | 0.5\% | 349,753 | 1.5\% |
| Model Area Total | 2,281,503 | 1.5\% | 2,454,081 | 1.7\% | 2,663,732 | 1.5\% | 2,875,506 | 1.4\% | 3,088,108 | 1.2\% | 3,278,757 | 1.1\% | 3,459,637 | 1.4\% |
| Note: MRM15v1.1 socioeconomic data for 2010 were derived from the 2010 U.S. Census. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

The official base year for the MRM15v1.1 is 2010 and serves as a fixed point from which growth is measured in this analysis. The base year contains actual data from the US Census. Since the Project is located within Mecklenburg County and Union County, labels for these counties are shown in italics for emphasis.

As seen in Table 4.1, the unadjusted 2010 total population in the 12 -county region amounted to nearly 2.28 million residents. The 2010 population in Mecklenburg and Union Counties was 920,000 and 201,000, respectively. These levels are forecast to increase to 3.59 million in the 12 -county region, 1.49 million in Mecklenburg County, and 341,000 in Union County. An additional 1.31 million residents in the region over the 30-year period translates to an average annual growth rate of approximately 1.5 percent. Mecklenburg County is expected to add 572,000 residents over the same 30-year period, resulting in an average annual growth rate of approximately 1.6 percent. The corresponding population increase expected for Union County is 140,000, or an average annual increase of 1.8 percent.

When compared with region-wide MRM15v1.1 projections, the adjusted population forecast shows approximately 28,000 fewer residents in 2015 and 130,000 fewer by 2040. Within Mecklenburg County, the adjusted projections forecast approximately 2,000 additional residents in 2015 and 19,000 fewer in 2040. Within Union County, the adjusted projections forecast approximately 9,000 fewer residents in 2015 and 18,000 fewer in 2040.

### 4.1.3 Model Area Households

Table 4.2 presents MRM15v1.1 household projections by county for each model year. The upper portion of Table 4.2 shows the original data set. The lower portion shows the adjusted data set.

The unadjusted data set for 2010 shows 875,000 households region-wide, 362,000 in Mecklenburg County, and 68,000 in Union County. By 2040, households are projected to total about 1.40 million region-wide, 595,000 in Mecklenburg County, and 115,000 in Union County. This represents an increase of 520,000 households region-wide resulting in an average annual increase of 1.6 percent. Mecklenburg County is expected to gain about 232,000 households over the same 30-year period, resulting in an average annual growth rate of approximately 1.7 percent. Households within Union County are forecast to increase by 47,000, representing an average annual increase of approximately 1.8 percent.

When compared with region-wide MRM15v1.1 projections, the adjusted forecast shows approximately 10,000 fewer households in 2015 and 51,000 fewer by 2040. Within Mecklenburg County, the adjusted projections forecast approximately 1,000 additional households in 2015 and 8,000 fewer in 2040. Within Union County, the adjusted projections forecast approximately 3,000 fewer households in 2015 and 7,000 fewer in 2040.

### 4.1.4 Model Area Employment

Table 4.3 presents MRM15v1.1 employment projections by county for each model year. Growth rates for each 5-year interval are calculated using AAPC. No adjustments were made to the employment projections contained within the model.
Table 4.2
Model Area Household Forecasts by County - MRM15v1.1 and Adjusted Data


[^0] AAPC is an abbreviation for Average Annual Percent Change.
Table 4.3

| County | MRM15v1.1 Data |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AAPC |  |  | AAPC |  | AAPC |  | AAPC |  | AAPC |  | AAPC |  | $\begin{gathered} \text { AAPC } \\ 2010-40 \end{gathered}$ |
|  | 2010 | 2010-15 | 2015 | 2015-20 | 2020 | 2020-25 | 2025 | 2025-30 | 2030 | 2030-35 | 2035 | 2035-40 | 2040 |  |
| Cabarrus | 69,595 | 2.9\% | 80,341 | 2.6\% | 91,409 | 2.2\% | 101,930 | 2.0\% | 112,729 | 1.9\% | 123,594 | 1.6\% | 133,988 | 2.2\% |
| Catawba | 1,756 | 2.5\% | 1,986 | 1.9\% | 2,179 | 1.0\% | 2,289 | 1.0\% | 2,405 | 1.2\% | 2,551 | 1.1\% | 2,698 | 1.4\% |
| Cleveland | 34,079 | 1.0\% | 35,858 | 0.8\% | 37,364 | 0.8\% | 38,826 | 0.6\% | 39,991 | 0.9\% | 41,805 | 0.7\% | 43,283 | 0.8\% |
| Gaston | 79,116 | 1.6\% | 85,548 | 1.4\% | 91,581 | 1.0\% | 96,414 | 0.9\% | 100,856 | 0.8\% | 105,215 | 0.7\% | 109,202 | 1.1\% |
| Iredell | 68,658 | 3.0\% | 79,590 | 2.5\% | 89,976 | 1.1\% | 94,969 | 0.9\% | 99,406 | 1.3\% | 105,845 | 1.1\% | 111,762 | 1.6\% |
| Lincoln | 25,101 | 1.1\% | 26,526 | 0.9\% | 27,700 | 1.0\% | 29,159 | 0.8\% | 30,396 | 1.2\% | 32,208 | 1.0\% | 33,806 | 1.0\% |
| Mecklenburg | 689,849 | 2.2\% | 768,304 | 1.9\% | 843,257 | 1.3\% | 899,099 | 1.1\% | 951,356 | 1.4\% | 1,017,632 | 1.2\% | 1,080,218 | 1.5\% |
| Rowan | 53,475 | 0.7\% | 55,432 | 0.7\% | 57,488 | 0.6\% | 59,168 | 0.6\% | 60,856 | 0.6\% | 62,812 | 0.5\% | 64,474 | 0.6\% |
| Stanly | 25,106 | 1.1\% | 26,559 | 0.8\% | 27,652 | 1.5\% | 29,842 | 1.2\% | 31,633 | 1.7\% | 34,476 | 1.3\% | 36,774 | 1.3\% |
| Union | 73,735 | 3.1\% | 85,974 | 1.4\% | 92,231 | 1.2\% | 97,759 | 1.1\% | 103,282 | 1.4\% | 110,518 | 1.1\% | 116,645 | 1.5\% |
| Lancaster | 4,210 | 16.7\% | 9,105 | 0.4\% | 9,281 | 0.3\% | 9,407 | 0.6\% | 9,672 | 0.6\% | 9,948 | 1.1\% | 10,494 | 3.1\% |
| York | 85,157 | 5.5\% | 111,318 | 1.4\% | 119,197 | 1.1\% | 125,836 | 1.1\% | 132,592 | 1.2\% | 140,482 | 1.1\% | 148,699 | 1.9\% |
| Model Area Total | 1,209,837 | 2.5\% | 1,366,541 | 1.7\% | 1,489,315 | 1.2\% | 1,584,698 | 1.1\% | 1,675,174 | 1.3\% | 1,787,086 | 1.1\% | 1,892,043 | 1.5\% |

The unadjusted data set shows total employment in the model region at 1.21 million in 2010. The majority of these jobs, nearly 690,000, were in Mecklenburg County. Total employment in Union County in 2010 was approximately 74,000 . Mecklenburg County, at 1.08 million jobs, is projected to remain the largest employment engine in the region in 2040. Union County employment is forecast to reach about 117,000 by 2040 .

### 4.1.5 Model Area Summary of Adjustments

Table 4.4 presents a summary of adjustments for population, households and employment on a region-wide basis. Differences between the original MRM15v1.1 data set and the adjusted data set are presented on an absolute and percent basis for all model years. Growth rates for the 30-year period from 2010 to 2040 are also provided for comparison.

Region-wide, adjustments to 2015 population and household projections totaled negative 1.1 percent and negative 1.0 percent, respectively. The downward adjustments made to population and household forecasts increase over time. By 2040, adjustments to population and household forecasts totaled negative 3.6 percent.

| Economic Input | 2010 | Table 4.4 <br> Model Area Summary of Adjustments |  |  |  |  |  | $\begin{aligned} & \text { AAPC } \\ & 2010-40 \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Forecast Year |  |  |  |  |  |  |
|  |  | 2015 | 2020 | 2025 | 2030 | 2035 | 2040 |  |
| Population |  |  |  |  |  |  |  |  |
| MRM15v1.1 Data | 2,281,503 | 2,481,853 | 2,703,407 | 2,932,549 | 3,166,184 | 3,374,484 | 3,589,203 | 1.5\% |
| Adjusted Data | 2,281,503 | 2,454,081 | 2,663,732 | 2,875,506 | 3,088,108 | 3,278,757 | 3,459,637 | 1.4\% |
| Difference |  | -27,772 | -39,675 | -57,043 | -78,076 | -95,727 | -129,566 |  |
| Percent Difference |  | -1.1\% | -1.5\% | -1.9\% | -2.5\% | -2.8\% | -3.6\% |  |
| Households |  |  |  |  |  |  |  |  |
| MRM15v1.1 Data | 875,191 | 952,792 | 1,035,944 | 1,122,383 | 1,215,624 | 1,299,389 | 1,395,639 | 1.6\% |
| Adjusted Data | 875,191 | 942,923 | 1,021,072 | 1,100,538 | 1,185,394 | 1,261,918 | 1,344,871 | 1.4\% |
| Difference |  | -9,869 | -14,872 | -21,845 | -30,230 | -37,471 | -50,768 |  |
| Percent Difference |  | -1.0\% | -1.4\% | -1.9\% | -2.5\% | -2.9\% | -3.6\% |  |
| Employment |  |  |  |  |  |  |  |  |
| MRM15v1.1 Data | 1,209,837 | 1,366,541 | 1,489,315 | 1,584,698 | 1,675,174 | 1,787,086 | 1,892,043 | 1.5\% |
| Adjusted Data | 1,209,837 | 1,366,541 | 1,489,315 | 1,584,698 | 1,675,174 | 1,787,086 | 1,892,043 | 1.5\% |
| Difference |  | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Percent Difference |  | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% |  |

Note: MRM15v1.1 socioeconomic data for 2010 were derived from the 2010 U.S. Census.
AAPC is an abbreviation for Average Annual Percent Change.

### 4.2 Study Area Adjustments

### 4.2.1 Study Area Boundaries

The MRM15v1.1 is divided into 3,439 geographic units called Traffic Analysis Zones (TAZs). Of those, 211 TAZs are in the vicinity of US 74 and the proposed Monroe Expressway. These 211 TAZs constitute the study area. The majority of the study area TAZs are in Union County. The western-most TAZs in the study area are in Mecklenburg County. The 211 study area TAZs have been grouped into

19 superzones for reporting purposes. These 19 superzones are numbered and depicted in relation to the proposed Monroe Expressway in Figure 4.1. The following sections describe the MRM15v1.1 forecast population, number of households, and employment in the study area compared to the adjusted forecasts.

### 4.2.2 Study Area Population

Table 4.5 presents the study area population projections by superzone for each model year. The upper portion of Table 4.5 shows the original data set. The lower portion shows the adjusted data set.

In the unadjusted data set, Superzone 2 (located near I-485 and to the north of the proposed Monroe Expressway) is expected to have the most rapid growth in population between 2010 and 2040 (averaging 3.0 percent per year). Population in the adjacent Superzone 3 is forecast to grow at an average 2.5 percent per year over the same time period. In absolute terms, population in Superzone 13 and Superzone 14 is expected to see the greatest increase between 2010 and 2040. Both of these zones are located near I-485 and south of US 74.

Within the adjusted data set, the most notable changes to population projections were made to superzones on the eastern end of the study area. Population projections for 2015 in Superzone 7 and Superzone 19 were reduced by 22.1 percent and 19.9 percent, respectively. Population projections for 2015 in Superzone 6 and Superzone 18 were reduced by 12.2 percent and 10.8 percent, respectively. Similar downward adjustments to these eastern-most superzones were made for all forecast years.

Figure 4.2 visually portrays the total forecasted change in population in the study area in the adjusted data set between 2010 and 2040 by TAZ (the smallest geographic unit of analysis in the model). It is apparent that forecast population growth is smallest in TAZs located east of Monroe. Larger increases in population growth are forecast to occur towards the western side of the study area, both to the north and south of US 74.

### 4.2.3 Study Area Households

Table 4.6 presents study area household projections by superzone for each model year. The upper portion of Table 4.6 shows the original data set. The lower portion shows the adjusted data set.

Unadjusted study area projections for number of households mirror population projections. Superzone 2 is anticipated to have the highest annual rate of growth in number of households, averaging 2.9 percent per year from 2010 to 2040 . Households in Superzone 3 are forecast to increase by an average 2.6 percent per year during the same time period. These superzones are located in the western portion of the study area. The largest increase in the number of households is anticipated to occur in Superzones 13, 14 and 2. These superzones are all located in the western half of the study area.

Within the adjusted data set, the most notable changes to household projections were made to superzones on the eastern end of the study area. Household projections in Superzones 7 and 19 were reduced by more than 20 percent in all forecast years, and reduced by more than 10 percent in Superzones 6 and 18 in all forecast years.

Table 4.5
Study Area Population Forecast by Superzone - MRM15v1.1 and Adjusted Data

| Superzone | Data |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AAPC |  | AAPC |  | AAPC |  | AAPC |  | AAPC |  | AAPC |  | AAPC |  |
|  | 2010 | 2010-15 | 2015 | 2015-20 | 2020 | 2020-25 | 2025 | 2025-30 | 2030 | 2030-35 | 2035 | 2035-40 | 2040 | 2010-40 |
| 1 | 9,919 | 2.0\% | 10,969 | 1.6\% | 11,888 | 1.6\% | 12,871 | 1.5\% | 13,853 | 1.3\% | 14,799 | 1.2\% | 15,741 | 1.6\% |
| 2 | 5,387 | 4.2\% | 6,617 | 7.8\% | 9,649 | 1.6\% | 10,458 | 1.5\% | 11,277 | 1.5\% | 12,119 | 1.3\% | 12,958 | 3.0\% |
| 3 | 2,151 | 3.0\% | 2,493 | 5.1\% | 3,196 | 2.1\% | 3,549 | 1.9\% | 3,892 | 1.7\% | 4,224 | 1.5\% | 4,554 | 2.5\% |
| 4 | 2,209 | 1.4\% | 2,366 | 1.1\% | 2,495 | 1.5\% | 2,688 | 1.4\% | 2,875 | 1.2\% | 3,057 | 1.2\% | 3,238 | 1.3\% |
| 5 | 2,762 | 1.8\% | 3,016 | 1.4\% | 3,228 | 1.9\% | 3,547 | 1.7\% | 3,856 | 1.5\% | 4,157 | 1.4\% | 4,455 | 1.6\% |
| 6 | 2,304 | 3.3\% | 2,712 | 1.3\% | 2,899 | 1.3\% | 3,090 | 1.1\% | 3,270 | 1.0\% | 3,443 | 1.0\% | 3,612 | 1.5\% |
| 7 | 621 | 5.6\% | 814 | 2.0\% | 897 | 1.6\% | 972 | 1.4\% | 1,041 | 1.2\% | 1,106 | 1.1\% | 1,168 | 2.1\% |
| 8 | 12,871 | 1.7\% | 13,988 | 0.8\% | 14,550 | 1.3\% | 15,551 | 1.3\% | 16,550 | 1.2\% | 17,560 | 1.1\% | 18,564 | 1.2\% |
| 9 | 5,033 | 1.7\% | 5,470 | 1.3\% | 5,830 | 1.8\% | 6,367 | 1.6\% | 6,884 | 1.4\% | 7,387 | 1.3\% | 7,883 | 1.5\% |
| 10 | 4,417 | 1.1\% | 4,657 | 0.8\% | 4,850 | 1.2\% | 5,139 | 1.1\% | 5,417 | 1.0\% | 5,690 | 0.9\% | 5,957 | 1.0\% |
| 11 | 4,860 | 0.0\% | 4,851 | 0.9\% | 5,065 | 1.4\% | 5,422 | 1.3\% | 5,788 | 1.2\% | 6,156 | 1.2\% | 6,520 | 1.0\% |
| 12 | 15,388 | 0.8\% | 16,031 | 0.8\% | 16,669 | 1.1\% | 17,563 | 1.0\% | 18,451 | 0.8\% | 19,212 | 0.8\% | 19,970 | 0.9\% |
| 13 | 21,515 | 1.9\% | 23,602 | 1.0\% | 24,750 | 1.9\% | 27,245 | 1.8\% | 29,731 | 1.3\% | 31,746 | 1.2\% | 33,750 | 1.5\% |
| 14 | 10,683 | 3.0\% | 12,367 | 2.5\% | 13,992 | 1.7\% | 15,191 | 1.5\% | 16,378 | 1.4\% | 17,567 | 1.3\% | 18,753 | 1.9\% |
| 15 | 11,222 | 2.3\% | 12,562 | -0.1\% | 12,511 | 1.9\% | 13,778 | 1.8\% | 15,044 | 1.7\% | 16,330 | 1.5\% | 17,610 | 1.5\% |
| 16 | 15,506 | 1.2\% | 16,464 | 1.0\% | 17,298 | 1.3\% | 18,408 | 1.1\% | 19,473 | 1.0\% | 20,503 | 1.0\% | 21,524 | 1.1\% |
| 17 | 9,415 | 1.5\% | 10,136 | 1.9\% | 11,142 | 1.1\% | 11,793 | 1.1\% | 12,429 | 1.0\% | 13,051 | 0.9\% | 13,658 | 1.2\% |
| 18 | 2,578 | 2.1\% | 2,854 | 1.6\% | 3,088 | 1.9\% | 3,392 | 1.7\% | 3,692 | 1.6\% | 3,988 | 1.4\% | 4,281 | 1.7\% |
| 19 | 1,413 | 4.9\% | 1,798 | 1.7\% | 1,959 | 1.4\% | 2,105 | 1.3\% | 2,240 | 1.1\% | 2,366 | 1.0\% | 2,489 | 1.9\% |
| Study Area Total | 140,254 | 1.9\% | 153,767 | 1.5\% | 165,956 | 1.5\% | 179,129 | 1.4\% | 192,141 | 1.3\% | 204,461 | 1.2\% | 216,685 | 1.5\% |
|  | Adjusted Data |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | AAPC |  | AAPC |  |  | AAPC | AAPC |  | AAPC |  | AAPC |  | AAPC |  |
| Superzone | 2010 | 2010-15 | 2015 | 2015-20 | 2020 | 2020-25 | 2025 | 2025-30 | 2030 | 2030-35 | 2035 | 2035-40 | 2040 | 2010-40 |
| 1 | 9,919 | 2.5\% | 11,243 | 1.6\% | 12,187 | 1.6\% | 13,198 | 1.5\% | 14,207 | 1.3\% | 15,188 | 1.2\% | 16,158 | 1.6\% |
| 2 | 5,387 | 5.6\% | 7,061 | 8.4\% | 10,560 | 1.5\% | 11,393 | 1.4\% | 12,238 | 1.4\% | 13,103 | 1.3\% | 13,966 | 3.2\% |
| 3 | 2,151 | 2.8\% | 2,468 | 4.1\% | 3,011 | 2.1\% | 3,343 | 1.9\% | 3,664 | 1.7\% | 3,977 | 1.5\% | 4,289 | 2.3\% |
| 4 | 2,209 | 0.6\% | 2,276 | 1.1\% | 2,401 | 1.5\% | 2,585 | 1.3\% | 2,761 | 1.2\% | 2,934 | 1.1\% | 3,105 | 1.1\% |
| 5 | 2,762 | 0.6\% | 2,840 | 1.4\% | 3,038 | 1.9\% | 3,333 | 1.7\% | 3,619 | 1.5\% | 3,897 | 1.4\% | 4,172 | 1.4\% |
| 6 | 2,304 | 0.7\% | 2,381 | 1.3\% | 2,539 | 1.2\% | 2,701 | 1.1\% | 2,854 | 1.0\% | 3,003 | 0.9\% | 3,148 | 1.0\% |
| 7 | 621 | 0.4\% | 634 | 1.9\% | 697 | 1.6\% | 753 | 1.3\% | 804 | 1.2\% | 853 | 1.1\% | 900 | 1.2\% |
| 8 | 12,871 | 0.8\% | 13,394 | 0.8\% | 13,940 | 1.3\% | 14,890 | 1.2\% | 15,838 | 1.2\% | 16,799 | 1.1\% | 17,756 | 1.1\% |
| 9 | 5,033 | 0.4\% | 5,141 | 1.3\% | 5,475 | 1.7\% | 5,969 | 1.5\% | 6,444 | 1.4\% | 6,907 | 1.3\% | 7,366 | 1.3\% |
| 10 | 4,417 | 0.2\% | 4,460 | 0.8\% | 4,644 | 1.2\% | 4,924 | 1.1\% | 5,192 | 1.0\% | 5,457 | 0.9\% | 5,713 | 0.9\% |
| 11 | 4,860 | -1.1\% | 4,593 | 0.9\% | 4,792 | 1.4\% | 5,126 | 1.3\% | 5,472 | 1.2\% | 5,818 | 1.1\% | 6,160 | 0.8\% |
| 12 | 15,388 | 0.3\% | 15,582 | 0.8\% | 16,181 | 1.0\% | 16,976 | 0.9\% | 17,765 | 0.8\% | 18,465 | 0.7\% | 19,159 | 0.7\% |
| 13 | 21,515 | 1.4\% | 23,098 | 0.9\% | 24,103 | 1.7\% | 26,161 | 1.5\% | 28,220 | 1.2\% | 29,989 | 1.1\% | 31,742 | 1.3\% |
| 14 | 10,683 | 1.9\% | 11,723 | 2.3\% | 13,151 | 1.7\% | 14,296 | 1.5\% | 15,432 | 1.4\% | 16,569 | 1.3\% | 17,703 | 1.7\% |
| 15 | 11,222 | 1.3\% | 11,987 | -0.1\% | 11,955 | 1.9\% | 13,126 | 1.7\% | 14,291 | 1.6\% | 15,471 | 1.5\% | 16,650 | 1.3\% |
| 16 | 15,506 | 0.5\% | 15,868 | 1.0\% | 16,664 | 1.2\% | 17,717 | 1.1\% | 18,725 | 1.0\% | 19,698 | 1.0\% | 20,660 | 1.0\% |
| 17 | 9,415 | 0.2\% | 9,519 | 1.7\% | 10,346 | 1.1\% | 10,940 | 1.0\% | 11,519 | 1.0\% | 12,085 | 0.9\% | 12,634 | 1.0\% |
| 18 | 2,578 | -0.2\% | 2,546 | 1.6\% | 2,752 | 1.8\% | 3,012 | 1.7\% | 3,272 | 1.5\% | 3,529 | 1.4\% | 3,781 | 1.3\% |
| 19 | 1,413 | 0.4\% | 1,441 | 1.6\% | 1,563 | 1.4\% | 1,679 | 1.3\% | 1,787 | 1.1\% | 1,888 | 1.1\% | 1,990 | 1.1\% |
| Study Area Total | 140,254 | 1.1\% | 148,255 | 1.5\% | 159,999 | 1.5\% | 172,122 | 1.4\% | 184,104 | 1.2\% | 195,630 | 1.1\% | 207,052 | 1.3\% |



TOTAL CHANGE IN POPULATION BY TAZ



Figure 4.3 visually portrays the total forecasted change in households in the study area, within the adjusted data set, between 2010 and 2040 by TAZ. Growth in the number of households is lowest in virtually all the TAZs east of US 601. Pockets of more aggressive growth in households is forecast to occur in the western TAZs, both to the north and south of US 74.

### 4.2.4 Study Area Employment

Table 4.7 presents study area employment projections by superzone for each model year. No adjustments were made to study area employment projections contained within the model.

The most rapid growth in employment is expected in Superzone 2, Superzone 3 and Superzone 5, each of which is expected to increase at an average annual rate in excess of 3.0 percent between 2010 and 2040. In absolute terms, Superzone 13 is expected to add the greatest number of jobs (approximately 9,000 ) over the same 30-year period. This accounts for more than 20 percent of the total increase in employment in the study area between 2010 and 2040.

Figure 4.4 portrays the total forecasted change in employment in the study area, within the adjusted data set, between 2010 and 2040 by TAZ. Larger employment increases are generally forecast to occur along the US 74 corridor compared to the surrounding TAZs.

### 4.2.5 Study Area Summary

Table 4.8 presents a bottom-line summary of adjustments made to population, household and employment forecasts within the study area. Differences between the original MRM15v1.1 data set and the adjusted data set are presented on an absolute and percent basis for all model years. Growth rates for the 30-year period from 2010 to 2040 are also provided for comparison.

Adjustments to 2015 population and household projections within study-area TAZs totaled negative 3.6 percent and negative 3.8 percent, respectively. By 2040 , adjustments to population and household forecasts totaled negative 4.4 percent and negative 5.0 percent, respectively. The resultant impact on 30-year AAPC was a decrease from 1.5 percent to 1.3 percent for both population and households.


TOTAL CHANGE IN HOUSEHOLDS BY TAZ WITHIN THE ADJUSTED DATASET FROM 2010-2040
Table 4.7

| Superzone | MRM15v1.1 Data |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AAPC |  | AAPC |  |  | AAPC | AAPC |  |  | AAPC |  | AAPC |  | AAPC |
|  | 2010 | 2010-15 | 2015 | 2015-20 | 2020 | 2020-25 | 2025 | 2025-30 | 2030 | 2030-35 | 2035 | 2035-40 | 2040 | 2010-40 |
| 1 | 2,835 | 3.5\% | 3,359 | 3.0\% | 3,887 | 1.6\% | 4,200 | 1.5\% | 4,519 | 1.8\% | 4,933 | 1.5\% | 5,316 | 2.1\% |
| 2 | 880 | 10.5\% | 1,448 | 2.6\% | 1,649 | 1.8\% | 1,807 | 1.7\% | 1,970 | 2.0\% | 2,179 | 1.7\% | 2,369 | 3.4\% |
| 3 | 250 | 7.6\% | 361 | 2.4\% | 407 | 2.1\% | 451 | 2.0\% | 499 | 2.7\% | 570 | 1.6\% | 617 | 3.1\% |
| 4 | 306 | 5.2\% | 395 | 2.0\% | 437 | 1.7\% | 476 | 1.5\% | 513 | 2.2\% | 572 | 1.3\% | 610 | 2.3\% |
| 5 | 170 | 11.0\% | 287 | 2.3\% | 322 | 3.2\% | 377 | 2.0\% | 417 | 3.5\% | 495 | 1.6\% | 536 | 3.9\% |
| 6 | 1,862 | 2.9\% | 2,145 | 0.9\% | 2,248 | 1.2\% | 2,391 | 1.1\% | 2,520 | 1.3\% | 2,689 | 1.1\% | 2,835 | 1.4\% |
| 7 | 129 | 2.1\% | 143 | 0.8\% | 149 | 2.6\% | 169 | 1.8\% | 185 | 2.3\% | 207 | 1.8\% | 226 | 1.9\% |
| 8 | 7,458 | 3.4\% | 8,798 | 1.3\% | 9,375 | 1.2\% | 9,943 | 1.0\% | 10,450 | 1.2\% | 11,066 | 1.0\% | 11,637 | 1.5\% |
| 9 | 3,278 | 1.9\% | 3,596 | 0.8\% | 3,749 | 1.4\% | 4,014 | 1.0\% | 4,215 | 1.2\% | 4,473 | 1.0\% | 4,710 | 1.2\% |
| 10 | 7,399 | 3.0\% | 8,579 | 0.9\% | 8,977 | 1.1\% | 9,505 | 0.8\% | 9,871 | 0.9\% | 10,318 | 0.8\% | 10,728 | 1.2\% |
| 11 | 2,110 | 3.9\% | 2,551 | 1.1\% | 2,701 | 1.6\% | 2,918 | 1.2\% | 3,090 | 1.3\% | 3,304 | 1.2\% | 3,499 | 1.7\% |
| 12 | 10,233 | 1.8\% | 11,169 | 1.6\% | 12,077 | 0.9\% | 12,660 | 0.9\% | 13,213 | 1.1\% | 13,923 | 0.9\% | 14,597 | 1.2\% |
| 13 | 9,140 | 6.3\% | 12,384 | 1.9\% | 13,637 | 1.5\% | 14,655 | 1.4\% | 15,672 | 1.6\% | 16,965 | 1.4\% | 18,177 | 2.3\% |
| 14 | 3,991 | 3.0\% | 4,634 | 1.2\% | 4,920 | 1.0\% | 5,174 | 1.0\% | 5,430 | 1.2\% | 5,754 | 1.0\% | 6,043 | 1.4\% |
| 15 | 5,792 | 3.2\% | 6,773 | 1.3\% | 7,210 | 1.5\% | 7,764 | 1.1\% | 8,213 | 1.3\% | 8,760 | 1.1\% | 9,273 | 1.6\% |
| 16 | 8,826 | 2.4\% | 9,942 | 1.1\% | 10,524 | 1.1\% | 11,130 | 1.0\% | 11,692 | 1.2\% | 12,419 | 1.0\% | 13,061 | 1.3\% |
| 17 | 8,475 | 1.9\% | 9,310 | 1.6\% | 10,064 | 0.7\% | 10,400 | 1.0\% | 10,939 | 1.2\% | 11,632 | 1.0\% | 12,238 | 1.2\% |
| 18 | 1,119 | 3.9\% | 1,355 | 2.7\% | 1,547 | 1.1\% | 1,633 | 1.6\% | 1,768 | 2.0\% | 1,950 | 1.5\% | 2,101 | 2.1\% |
| 19 | 968 | 1.9\% | 1,061 | 1.1\% | 1,120 | 1.0\% | 1,177 | 0.9\% | 1,232 | 1.4\% | 1,318 | 0.9\% | 1,380 | 1.2\% |
| Study Area Total | 75,221 | 3.3\% | 88,290 | 1.5\% | 95,000 | 1.2\% | 100,844 | 1.1\% | 106,408 | 1.3\% | 113,527 | 1.1\% | 119,953 | 1.6\% |
| Note: MRM15v1.1 socioeconomic data for 2010 were derived from the 2010 U.S. Census. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |



TOTAL CHANGE IN EMPLOYMENT BY TAZ

Table 4.8
Study Area Summary of Adjustments

| Economic Input | 2010 | Forecast Year |  |  |  |  |  | $\begin{gathered} \text { AAPC } \\ 2010-40 \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 2015 | 2020 | 2025 | 2030 | 2035 | 2040 |  |
| Population |  |  |  |  |  |  |  |  |
| MRM15v1.1 Data | 140,254 | 153,767 | 165,956 | 179,129 | 192,141 | 204,461 | 216,685 | 1.5\% |
| Adjusted Data | 140,254 | 148,255 | 159,999 | 172,122 | 184,104 | 195,630 | 207,052 | 1.3\% |
| Difference |  | -5,512 | -5,957 | -7,007 | -8,037 | -8,831 | -9,633 |  |
| Percent Difference |  | -3.6\% | -3.6\% | -3.9\% | -4.2\% | -4.3\% | -4.4\% |  |
| Households |  |  |  |  |  |  |  |  |
| MRM15v1.1 Data | 48,536 | 53,247 | 57,376 | 62,187 | 66,901 | 71,322 | 75,638 | 1.5\% |
| Adjusted Data | 48,536 | 51,222 | 55,164 | 59,533 | 63,812 | 67,902 | 71,891 | 1.3\% |
| Difference |  | -2,025 | -2,212 | -2,654 | -3,089 | -3,420 | -3,747 |  |
| Percent Difference |  | -3.8\% | -3.9\% | -4.3\% | -4.6\% | -4.8\% | -5.0\% |  |
| Employment |  |  |  |  |  |  |  |  |
| MRM15v1.1 Data | 75,221 | 88,290 | 95,000 | 100,844 | 106,408 | 113,527 | 119,953 | 1.6\% |
| Adjusted Data | 75,221 | 88,290 | 95,000 | 100,844 | 106,408 | 113,527 | 119,953 | 1.6\% |
| Difference |  | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Percent Difference |  | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% |  |
| Note: MRM15v1.1 socioeconomic data for 2010 were derived from the 2010 U.S. Census. AAPC is an abbreviation for Average Annual Percent Change. |  |  |  |  |  |  |  |  |

## Chapter 5

## Model Calibration

The traffic and toll revenue potential of the Monroe Expressway was analyzed using the Metrolina Regional Travel Demand Model 2015 Version 1.1 (MRM15v1.1 or MRM). In the fall of 2015, CDM Smith obtained a working version of the MRM15v1.1 and the model documentation. The official base year of the MRM15v1.1 is 2010, which incorporates 2010 US Census data. The model forecast years include 2015, 2025, 2030 and 2040. CDM Smith used the 2015 model year as the new base year, and calibrated to 2015 conditions. The calibration effort focused on the detailed study area which is shown in green shading in Figure 3.1. and highlighted in Figure 4.1.

The MRM15v1.1 was calibrated first for traffic volumes, then for observed travel speeds and finally for observed sample trip distances on US 74 within the study corridor. Each of these variables impact each other, thus necessitating an iterative process to achieve the desired results for each target metric.

Calibration involves modifying the model's internal parameters so that the model reflects current observed conditions. This chapter provides a summary of the degree to which the output of the baseyear travel demand model matches observed data.

Actual data metrics used for calibration included 1) available traffic counts from NCDOT, 2) traffic counts conducted by The Traffic Group, 3) INRIX travel time and speed data, and 4) sample trip distance distributions, primarily on US 74, obtained from ALPR surveys. Details of the data collection efforts and a summary of the collected data described in Chapter 2 of this report and in the Monroe Expressway - Data Collection memorandum dated December 18, 2015.

In order to demonstrate that the 2015 base-year model sufficiently represents existing conditions, the following comparisons were made between model results and observed field data:

1. Model traffic-volume output compared to traffic counts at locations along seven screenlines;
2. Model traffic-volume output compared to traffic counts located on US 74;
3. Model travel-speed output compared to 2015 INRIX travel speed data; and
4. Comparisons of select trip lengths on US 74 derived from ALPR surveys compared to model output.

### 5.1 Traffic Volume Calibration Results

One assessment of model calibration is whether the total assigned screenline volume compares well with actual traffic counts. The level of difference between counts and volumes may vary between individual links, but if the total assigned volume crossing the screenline is reasonably close to actual counts, this is an indication that the general level of traffic and travel patterns is fairly well represented by the model.

As mentioned previously, the model calibration was performed at 2015 levels. Calibrated model was compared to average weekday traffic volumes as the model is a weekday model. An evaluation was conducted by comparing traffic counts with 2015 model output. Various criteria, including minimizing the root mean square error (RMSE), volume count comparisons and GEH statistic (a commonly used
method for comparing two sets of traffic volumes) were used to test model refinements as subsequent calibration tests were performed.

Figure 5.1 shows the location of traffic count screenlines within the detailed study area. As shown in the figure, seven screenlines were developed for this study. All of these screenlines are drawn perpendicular to US 74, capturing roads parallel to the Monroe Expressway. The Monroe Expressway will pull traffic from these roads. Screenlines 2 through 6 pass through the Monroe Expressway project alignment.

Base year calibration adjustments were conducted to obtain reasonable matches between the observed and estimated traffic volumes at screenline count locations. Table $\mathbf{5 . 1}$ provides a comparison of unadjusted MRM screenline volumes and corresponding traffic counts. The first column shows the combined average weekday traffic (AWDT) count across each screenline, while the second and third columns show the pre-and-post calibration MRM results, respectively. Despite an overall difference of only 1.4 percent, four of the seven screenline volume totals differed from corresponding traffic counts by more than 20.0 percent within the pre-calibration results. The calibration process drastically improves this variance and brings the greatest difference down to 1.7 percent. The combined total volume across all screenlines is approximately 0.1 percent above the average weekday counts within the post-calibration results.

Table 5.1
Comparison of Assigned Model Volumes with Actual Counts by Screenline Location

| Screenline ID | Count | Pre-Calibration |  | Post-Calibration |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Model Output | Difference from Data | Model Output | Difference from Data |
| 1 | 189,480 | 229,172 | 20.9\% | 188,993 | -0.3\% |
| 2 | 163,372 | 172,726 | 5.7\% | 162,634 | -0.5\% |
| 3 | 123,939 | 129,143 | 4.2\% | 123,907 | 0.0\% |
| 4 | 107,628 | 71,652 | -33.4\% | 109,408 | 1.7\% |
| 5 | 89,668 | 102,566 | 14.4\% | 89,886 | 0.2\% |
| 6 | 55,924 | 41,914 | -25.1\% | 55,748 | -0.3\% |
| 7 | 27,624 | 20,922 | -24.3\% | 27,722 | 0.4\% |
| Totals | 757,635 | 768,095 | 1.4\% | 758,300 | 0.1\% |

Figure 5.2 presents a scatter plot comparing post-calibration model volumes versus observed traffic volumes for each of the 154 count locations contained within the 7 screenlines. The comparisons are shown for the four time-periods modeled in this study - AM Peak, Midday, PM Peak and Overnight. These figures show traffic counts plotted against corresponding model output for each time period. Each scatterplot is fitted with a trend line, and the corresponding equation and $\mathrm{R}^{2}$ value is printed on each. Trend line factors close to 1.00 represent calibrations that are close to observed conditions. The regression line shows that the calibrated 2015 CDM Smith base-year model matches observed traffic closely at most locations, i.e., the plot of model volumes compared to observed traffic volumes is close to the $y=x$ line, and there are no extreme outliers. Overall the model appears to have a very reasonable $\mathrm{R}^{2}$, or "goodness-of-fit", greater than 0.99 .

Table 5.2 presents 2015 base-year volume calibration results for the 15 classification count locations (conducted by The Traffic Group) on US 74 and several other roads that are alternatives to US 74. The
location of each count station is portrayed in Figure 2.1 and described in Table 2.1. Stations 1 through 7 are particularly important as US 74 is the source of the majority of traffic that will shift to the Monroe Expressway. Traffic count volumes presented in Table 5.2 represent three-day weekday averages (Tuesday, Wednesday, Thursday). Pre-calibration results showed variances ranging from 23.9 percent to negative 80.7 percent between traffic counts and model output. Post-calibration results showed variances ranging from 1.2 percent to negative 1.9 percent, while 13 of the 15 locations had a variance of just plus or minus $0.5 \%$.

Table 5.2
Comparison of Assigned Model Volumes with Actual Counts at Classification Count Locations

| $\qquad$ | Road | Average <br> Weekday (1) | Pre-Calibration |  | Post-Calibration |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Model | Difference | Model | Difference |
|  |  |  | Output | from Data | Output | from Data |
| 1 | US 74 | 60,011 | 74,369 | 23.9\% | 60,736 | 1.2\% |
| 2 | US 74 | 48,637 | 59,421 | 22.2\% | 48,790 | 0.3\% |
| 3 | US 74 | 41,138 | 44,710 | 8.7\% | 41,232 | 0.2\% |
| 4 | US 74 | 45,634 | 40,737 | -10.7\% | 45,752 | 0.3\% |
| 5 | US 74 | 50,853 | 29,940 | -41.1\% | 50,914 | 0.1\% |
| 6 | US 74 | 29,810 | 15,716 | -47.3\% | 29,769 | -0.1\% |
| 7 | US 74 | 20,816 | 9,732 | -53.2\% | 20,799 | -0.1\% |
| 8 | Old Charlotte Highway | 26,904 | 24,833 | -7.7\% | 27,045 | 0.5\% |
| 9 | North Charlotte Avenue | 11,852 | 7,981 | -32.7\% | 11,630 | -1.9\% |
| 10 | Idlewild Road | 21,992 | 22,211 | 1.0\% | 21,993 | 0.0\% |
| 11 | Secrest Short Cut Road | 10,348 | 8,786 | -15.1\% | 10,393 | 0.4\% |
| 12 | Secrest Short Cut Road | 9,704 | 11,991 | 23.6\% | 9,703 | 0.0\% |
| 13 | NC 84 | 9,429 | 10,515 | 11.5\% | 9,385 | -0.5\% |
| 14 | NC 200 | 10,699 | 10,825 | 1.2\% | 10,702 | 0.0\% |
| 15 | NC 218 | 7,968 | 1,536 | -80.7\% | 7,945 | -0.3\% |
| (1) Average weekday taffic volumes are based on a three-day internal weekday average including Tuesday, Wednesday and Thursday. |  |  |  |  |  |  |

NCDOT Monroe Expressway Traffic and Toll Revenue Study



PM Peak


Midday Peak


Overnight


REGRESSION RESULTS FROM CALIBRATION

### 5.2 Travel Speed Calibration Results

In a well calibrated model, both traffic volumes and travel times must be accurately represented. This section compares model output with INRIX travel speeds on US 74, the primary alternative route to the proposed Monroe Expressway.

Table 5.3 presents a weekday travel time comparison between the INRIX data and model output for a through trip on US 74 between Forest Hills School Road in Marshville and I-485 in Mathews. Comparisons are shown for three modeled time periods - AM Peak, Midday and PM Peak. A before-and-after calibration comparison is included in Table 5.3 to present the impacts of the calibration effort. Similar to the travel time data presented in Table 5.3, travel speed comparisons are shown in Table 5.4. US 74 speeds in the model are a little higher than the INRIX data, except during the Midday in the westbound direction. This is a conservative assumption when evaluating the Monroe Expressway as the primary competitor to US 74. Reducing the speeds on US 74 down to the INRIX averages was found to push more volume off of US 74 than desired.

Table 5.3
Comparison of US 74 Observed and Model Travel Times (in minutes)

| Direction | Time <br> Period | INRIX <br> Data | Pre-Calibration |  | Post-Calibration |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Model Output | Difference from Data | Model Output | Difference from Data |
| Westbound | AM | 29.9 | 26.9 | -2.9 | 26.9 | -3.0 |
|  | MD | 26.2 | 26.4 | 0.2 | 26.9 | 0.7 |
|  | PM | 30.8 | 26.7 | -4.1 | 27.7 | -3.0 |
| Eastbound | AM | 26.9 | 26.8 | 0.0 | 27.0 | 0.2 |
|  | MD | 28.5 | 26.6 | -1.8 | 27.3 | -1.2 |
|  | PM | 31.2 | 27.6 | -3.7 | 28.1 | -3.2 |

Table 5.4
Comparison of US 74 Observed and Model Travel Speeds (in mph)

| Direction | Time Period | INRIX <br> Data | Pre-Calibration |  | Post-Calibration |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Model <br> Output | Difference from Data | Model <br> Output | Difference from Data |
| Westbound | AM | 38.0 | 42.6 | 4.6 | 42.6 | 4.6 |
|  | MD | 44.0 | 43.6 | -0.4 | 42.8 | -1.2 |
|  | PM | 37.0 | 43.1 | 6.1 | 41.5 | 4.5 |
| Eastbound | AM | 43.0 | 43.4 | 0.4 | 43.1 | 0.1 |
|  | MD | 40.0 | 43.7 | 3.7 | 42.7 | 2.7 |
|  | PM | 37.0 | 42.0 | 5.0 | 41.2 | 4.2 |

### 5.3 Automatic License Plate Recognition Survey

An ALPR survey was conducted for the primary purpose of identifying sample trip lengths along the study corridor, particularly on US 74. The ALPR survey was conducted at 12 locations from 5:30 AM to 7:00 PM on Thursday, October 22, 2015. These locations are shown in Figure 2.1. The ALPR survey effort consisted of recording license plate images by video camera and matching the plates across the survey locations to assist in determining weekday car and truck trip lengths on US 74 between Wingate and I-485. This data was beneficial to calibrating the model because motorists currently on US 74 are much more likely to use the Monroe Expressway for longer distance trips compared to very short trips, so trip length distributions may impact the travel demand on the Monroe Expressway. The collection efforts are described in Section 2.4.

Captured license-plate images were matched across survey stations and checked for reasonableness based on logical movements and travel time to identify unique trips. The results discussed in this section focus on US 74 during the peak travel periods. ALPR survey results for passenger cars during the AM Peak period on US 74 are summarized in the top box of Table 5.5. All movements reflect westbound trips along US 74. The station where a plate was first identified (the origin) is shown in the left column, while the last station where the same plate was identified (the destination) is shown across the top. The percent distribution of trips from each origin station are unique, thus, the sum of each row adds up to 100 percent. Each row describes movements that are identified as a percentage of the trips that started at a station and traveled far enough to be captured at a downstream station. For example, for passenger cars in the AM time period, 4 percent of trips with an origin at Station 7 went as far as Station 3 (but did not reach Station 2), another 4 percent had a destination that went as far as Station 2 (but did not reach Station 1), and 17 percent had a destination that went as far as Station 1, or beyond. Any trip that was captured at Station 7, but did not also pass through Stations 6, 5, 4, 3, 2, or 1, would not be represented in this table.

It is important to recognize that these matrices are not complete trip tables, as US 74 is not an access controlled road. US 74 has numerous access points. The trip distance matrices represent snapshots of specific locations, chosen to fall between major intersecting roads with US 74. However, the data was useful for adjusting trip distance distributions in the MRM.

As discussed previously, the 2015 MRM was initially calibrated to screenline traffic counts and counts along US 74 in the study corridor. Another series of calibration assignments were made to further calibrate to weekday travel speeds by time period. The calibration process resulted in changes to travel patterns within the model, including trip distance distributions. The final iterative calibration process involved re-adjusting trip distances where necessary.

Table 5.5 also contains the resulting passenger-car trip-distance distributions from the calibrated 2015 MRM for the AM Peak period. The resulting calibration shows that the number of the longest distance trips on US 74, from the model output, are underestimated compared to the survey data. For example, the survey data shows that about 17 percent of passenger cars entering Station 7 travel through station 1 (the longest possible trip), while the model output has only 11 percent of passenger car through trips. The longest distance trips are also underestimated for passenger cars entering from stations 6,5 , and 4 . Some intermediate distance trips are somewhat over represented in the model output. Examples of this condition includes passenger cars entering from Station 6 and traveling through Station 3. Some movements match exactly, such as movements that entered Station 5 or 4, and traveled through Station 2. Very short distance trips are less likely to divert to the Monroe Expressway.

Table 5.6 compares trip-distance distributions between the 2015 calibrated model and the ALPR survey for passenger cars during the PM Peak period. The comparison shows similar patterns to the AM Peak period, in that the longest distance trips are under-represented in the calibrated model and the intermediate distance trips tend to be over represented.

It is not expected that the calibrated model output will replicate the trip-distance data. US 74 actual travel patterns are complicated due to an unlimited number of access points, while the model is constrained to access at intersections and at a limited numbers of centroid connectors. In addition, calibrating to traffic volumes and travel speeds alter travel patterns in numerous ways. In summary, the model was calibrated to reasonable parameters, and reflects a conservative calibration in that it does not over represent the longest distance trips that are more likely to use the Monroe Expressway.

Table 5.5
US 74 Survey Results vs. MRM Trip Distance Patterns - AM Peak

| Entering At Westbound Survey Station |  | ALPR SURVEY |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| US 74 | 1 |  |  |  |  |  |  |  |
|  | 2 | 100\% |  |  |  |  |  |  |
|  | 3 | 60\% | 40\% |  |  |  |  |  |
|  | 4 | 44\% | 24\% | 32\% |  |  |  |  |
|  | 5 | 32\% | 16\% | 20\% | 32\% |  |  |  |
|  | 6 | 20\% | 10\% | 11\% | 18\% | 42\% | M/Ill\| |  |
|  | 7 | 17\% | 4\% | 4\% | 9\% | 17\% | 50\% |  |
|  |  |  |  |  |  |  |  |  |
| Entering At Westbound Survey Station |  | MRM POST-CALIBRATION |  |  |  |  |  |  |
|  |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| US 74 | 1 |  |  |  |  |  |  |  |
|  | 2 | 100\% |  |  |  |  |  |  |
|  | 3 | 45\% | 55\% |  |  |  |  |  |
|  | 4 | 22\% | 24\% | 54\% |  |  |  |  |
|  | 5 | 16\% | 16\% | 27\% | 41\% |  |  |  |
|  | 6 | 16\% | 16\% | 18\% | 21\% | 29\% |  |  |
|  | 7 | 11\% | 11\% | 11\% | 13\% | 15\% | 39\% |  |

Table 5.6
US 74 Survey Results vs. MRM Trip Distance Patterns - PM Peak

| Entering At Westbound Survey Station |  | ALPR SURVEY |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| US 74 | 1 |  |  |  |  |  |  |  |
|  | 2 | 100\% |  |  |  |  |  |  |
|  | 3 | 60\% | 40\% |  |  |  |  |  |
|  | 4 | 48\% | 26\% | 26\% |  | \#UIU! |  |  |
|  | 5 | 37\% | 14\% | 10\% | 40\% |  |  |  |
|  | 6 | 20\% | 8\% | 6\% | 19\% | 47\% |  |  |
|  | 7 | 23\% | 5\% | 3\% | 7\% | 21\% | 42\% |  |
|  |  |  |  |  |  |  |  |  |
| Entering At Westbound Survey Station |  | MRM POST-CALIBRATION |  |  |  |  |  |  |
|  |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| US 74 | 1 |  |  |  |  |  |  |  |
|  | 2 | 100\% |  |  |  |  |  |  |
|  | 3 | 46\% | 54\% |  |  |  |  |  |
|  | 4 | 29\% | 33\% | 39\% |  |  |  |  |
|  | 5 | 13\% | 18\% | 21\% | 47\% |  |  |  |
|  | 6 | 12\% | 12\% | 13\% | 21\% | 42\% |  |  |
|  | 7 | 8\% | 8\% | 8\% | 11\% | 17\% | 49\% |  |

## Chapter 6

## Traffic and Revenue Analysis

This chapter presents a summary of the traffic and revenue analysis conducted for the Monroe Expressway. In addition to an overview of the travel demand modeling process, this chapter also presents information about the regional highway improvement program, basic assumptions and modeling inputs, the toll sensitivity analysis, recommended toll rates, and the Base Condition traffic and toll revenue forecasts for the Monroe Expressway.

### 6.1 Analytical Methodology

The modeling process used to analyze the traffic and toll revenue potential of the Monroe Expressway is depicted in Figure 6.1. The following describes the major steps in the modeling process.

As described in Chapter 3, Section 3.1, the modeling process was initiated by obtaining the Metrolina Regional Model, specifically the MRM15v1.1. This model's base year is 2010, which reflects 2010 US Census data. The MRM supports future years 2015, 2025, 2030 and 2040. The MRM includes highway and transit networks and land-use and socio-economic data for each model year. Transit and vehicle trip tables are derived from the model.

CDM Smith reviewed and modified the highway network as described in Chapter 3. These modifications included disaggregating TAZs, changing centroid connections, and adding additional roads or road segments to the networks. A comparison was also made between the 2015 network and actual ground conditions observed in aerials available on the internet. These comparisons were made to validate the 2015 network in the study area against current conditions. No substantive changes to distances, access points, or number of lanes were required.

Data inputs necessary for calibrating the model and using the model to develop toll transaction and revenue forecasts was obtained and analyzed by CDM Smith. This data included:

1. Traffic count data;
2. Travel time and speed data;
3. License plate survey data;
4. Current values of time;
5. Current vehicle operating costs; and
6. ETC market share.

A major analytical component of the modeling process was the independent review of the underlying socioeconomic assumptions in the MRM. This was undertaken by Dr. Appold, an economist with recognized expertise in the area's land-use and socioeconomic profile. Adjustments to the socioeconomic data in the MRM were made by the economist for all forecast model years. After a review by CDM Smith, the economic adjustments were incorporated into the MRM model resulting in updated vehicle trip tables that reflected new growth forecasts.


CDOT: Charlotte Department of Transportation.

MODELING PROCESS FOR TOLL ROAD ANALYSIS

All the new data inputs and the updated vehicle trip tables were incorporated into the MRM forecast years (2015, 2025, 2030 and 2040). The next step was to calibrate the 2015 model to existing conditions in the study area. Calibration included comparing the model output against known traffic volumes and travel speeds on area roads. In addition, the trip distance distributions acquired by the ALPR survey were used in calibration to adjust model trip distances on US 74. The calibration results were described in Chapter 5. Changes in the model necessary to achieve calibration in 2015 were carried forward into the future year networks. Based on the revised model, a 2019 model year was created that contains the Monroe Expressway project coding, as the Expressway is assumed to open on January 1, 2019.

Toll sensitivity analysis was conducted for the opening year (2019) to determine the "optimal" toll rate that should be assessed. This was accomplished by running a series of toll assignments at progressively higher toll rates, and evaluating the resulting toll transactions and toll revenue on the Expressway. "Optimal" can mean different things; in this case, optimal refers to a point on the curve that nearly maximizes toll revenue, yet leaves some room to make some upward adjustments in rates should the need arise. Once the 2019 toll rates were identified, the toll rates were developed for the future years, assuming that the toll rates would be adjusted annually to keep up with inflation.

Once the toll rates were selected for 2019, for Electronic Toll Collection (ETC) and Video Toll Collection (VTC), a series of assignments were made for years 2019, 2025, 2030 and 2040 to develop the traffic and revenue estimates for the Monroe Expressway. These assignments were run for four time periods, by cars and trucks, and by two methods of payment (ETC and VTC). The assignments compared the travel time and distance for a trip path on the Monroe Expressway with a path on the best toll-free alternative route. The estimated traffic that would be expected to use the toll road is a function of 1) the travel time saved and the distance saved, 2) the assumed monetary value of these savings, and 3) the toll rate being tested in any given assignment. In general, as the total costs to use the proposed toll road increase, estimated usage of the toll road decreases. The model recognizes capacity constraints on roadways. Speeds for future-year forecasts are calculated based on volume to capacity ratios and reflect increasing congestion over time on both the proposed toll facility and existing toll free roads.

The toll diversion assignments result in forecasts of calendar-year average annual weekday tolled traffic by toll zone on the Monroe Expressway for the assignment years. Intermediate year traffic volumes were interpolated. Toll transactions for the years subsequent to the available model years were developed by assuming an annual increase in traffic based on decreasing the prior trend line.

Annual gross toll revenue estimates were developed from the average annual weekday transactions (AAWDTs) by annualizing the tolled traffic estimates to take into account weekend day traffic and holidays, and calculating the annual gross toll revenue. The traffic and toll revenue forecasts were developed for calendar years because the MRM model operates on a calendar year basis. The forecasts were then converted to the NCDOT fiscal year, which begins on July 1. The gross toll revenue does not include adjustments for uncollectible revenue or fee revenue associated with VTC. These adjustments were added as a last step based on actual experience from the Triangle Expressway, which has been in operation as a toll facility since January 3, 2012.

### 6.2 Modeling Inputs

This section describes key inputs to the model that influence the traffic and toll revenue analysis process.

### 6.2.1 Toll Collection

Toll collection for the Monroe Expressway is assumed to be consistent with the existing toll collection methods used on the Triangle Expressway. The Monroe Expressway will operate as an all-electronic tolling (AET) system. This system will allow motorists to pay their toll without stopping or slowing down. Instead of passing through a conventional toll booth, an overhead toll collection system will be mounted to a gantry, a structure elevated over the road. As the motorist passes under the gantry, either an ETC transponder is read and the toll is automatically charged to the transponder account, or a photo is taken of the license plate and the registered owner of the vehicle will receive a Bill By Mail (BBM) invoice in the mail. An AET system generally provides a safer, quicker, and more convenient experience for motorists.

On the Monroe Expressway, tolls will be collected via NC Quick Pass (NCDOT's ETC program) or by BBM (NCDOT's VTC Program). The NC Quick Pass program is currently interoperable with Georgia's Peach Pass, Florida's Sun Pass and E-ZPass. A map of states with NC Quick Pass interoperable ETC programs is shown in Figure 6.2. Throughout this study, ETC refers to NC Quick Pass and the other interoperable systems. This interoperability arrangement is assumed to continue through the forecast period of this study.

All vehicles will be permitted to use the Monroe Expressway. The following three toll classes will be implemented:

- Class 1 (2-axle vehicles): includes all two-axle vehicles regardless of the number of tires.
- Class 2 (3-axle vehicles): includes all three-axle vehicles including two-axle vehicles towing a single-axle trailer.
- Class 3 (4-or-more axle vehicles): includes all vehicles with four-or-more axles (4+) including two-axle vehicles towing a dual-axle trailer.

Motorists who pay with NC Quick Pass (or interoperable systems) will receive a 35 percent discount off of the BBM rates for all vehicle toll classes. All Class 2 toll rates will equal two times the Class 1 toll rate, and Class 3 toll rates will equal four times the Class 1 toll rate.

### 6.2.2 ETC Market Share

The assumed future market shares of ETC and BBM transactions is an important input into the traffic demand model. ETC transactions are at less "risk" of non-payment compared to BBM transactions, which are more prone to "leakage" or non-payment for a variety of reasons. CDM Smith developed ETC and BBM market shares for modeling years 2019, 2025, 2030 and 2040 based on actual experience from the Triangle Expressway and other AET facilities in the US.

Table 6.1 presents total annual weekday toll transactions by ETC and BBM on the Triangle Expressway from its opening year in 2012 through the second quarter of 2015. The ETC market share totaled 49.2 percent in 2012, 57.5 percent in 2013 and 58.1 percent in 2014. Through the second quarter of 2015, the ETC market share totaled 58.3 percent. It appears that the rate of increase in annual ETC market share after 2013 has been quite modest.

The Monroe Expressway is likely to be the second dedicated toll road in North Carolina in 2019, after the Triangle Expressway in the Raleigh area. The I-77 Express lanes project in Charlotte is expected to open in late 2017. Due to the small number of operating toll roads in the state, there won't likely be a CDM

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$\left.\begin{array}{l}\text { States Within the } \\
\text { E-ZPass Network } \\
\text { GA Peach Pass, } \\
\text { FL SunPass }\end{array}\right\}$ ETC Interoperable With

| SC Quick Pass |
| :--- |
| Not Interoperable With NC Quick Pass or E-ZPass Network |
| States Without Electronic Toll Collection |

large population of motorists with an NC Quick Pass Account. There will be a positive benefit from North Carolina's proximity to other states with toll facilities that use E-ZPass, as shown previously in Figure 6.2.

Table 6.1
Observed Weekday ETC Market Share on the Triangle Expressway

| Calendar Year | Number Of <br> Annual Weekday Transactions |  |  | Percent Market Share By Transactions |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | ETC | BBM | Total | ETC | BBM | Total |
| 2012 | 2,806,662 | 2,901,039 | 5,707,701 | 49.2 | 50.8 | 100.0 |
| 2013 (1) | 13,258,229 | 9,800,970 | 23,059,199 | 57.5 | 42.5 | 100.0 |
| 2014 | 17,801,006 | 12,849,303 | 30,650,309 | 58.1 | 41.9 | 100.0 |
| 2015 (2) | 10,378,659 | 7,431,990 | 17,810,649 | 58.3 | 41.7 | 100.0 |

1) The last section of the Triangle Expressway opened to toll traffic on 1/3/2013, from US 64 to NC 55.
2) Data through June 2015.

Source: North Carolina Turnpike Authority Operations Statistics Report

Future-year estimates of ETC and BBM annual weekday market shares for the Monroe Expressway are presented in Table 6.2 for modeling years 2019, 2025, 2030 and 2040. The estimated percent market shares were developed for Class 1 vehicles (2-axle vehicles) and combined Class 2 and 3 vehicles ( 3 and $4+$ axle vehicles). Weekday market shares are presented because the MRM model represents weekday traffic volumes.

The ETC market share for Class 1 vehicles is estimated to total about 59 percent in 2019, and increase to about 61, 64 and 68 percent in 2025, 2030 and 2040, respectively. These increases represent average annual increases of about 0.4 percentage points per year. Commercial vehicles (Classes 2 and 3 combined are anticipated to have higher ETC participation rates, starting at about 70 percent in 2019, and increasing to about 71,72 and 74 percent in 2025, 2030 and 2040, respectively.

Table 6.2
Assumed Annual Weekday ETC Market Share on the Monroe Expressway

| Calendar Year | Class 1 Vehicles |  |  | Class 2 and 3 Vehicles |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | ETC | BBM | Total | ETC | BBM | Total |
| 2019 (1) | 59.0 | 41.0 | 100.0 | 70.0 | 30.0 | 100.0 |
| 2025 | 61.5 | 38.5 | 100.0 | 71.0 | 29.0 | 100.0 |
| 2030 | 64.0 | 36.0 | 100.0 | 72.0 | 28.0 | 100.0 |
| 2040 | 68.0 | 32.0 | 100.0 | 74.0 | 26.0 | 100.0 |
| 1) Assumes the entire Monroe Expressway opens on January 1, 2019. |  |  |  |  |  |  |

### 6.2.3 Value of Time

Estimates of motorist Value of Time (VOT) were calculated based on median household income data in the MRM model. The data was available for each traffic analysis zone (TAZ) in the model. In aggregate, based on weighted trips for each TAZ in the model, the 2015 passenger-car VOT is estimated to equal $\$ 10.40$ per hour (or $\$ 0.173$ per minute) in the project corridor. Values of time for commercial vehicles were assumed to increase by a factor of 1.8 . The VOTs were inflated annually for use in future-year assignments based on forecast inflation rates described in Section 6.2.5.

### 6.2.4 Vehicle Operating Cost

Vehicle Operating Costs (VOC) take into account the wear and tear on a vehicle as expressed in maintenance costs, tires, and other variable costs such as oil and fuel. A passenger-car operating cost of $\$ 0.168$ per mile was estimated for 2015 based on the following data:

- The average cost of gasoline in the Lower Atlantic states provided by the Energy Information Administration.
- The average cost of tires and maintenance by passenger car vehicle type provided in the 2014 Your Driving Costs report published by AAA.
- The passenger-car vehicle type distribution for the State of North Carolina published in the 2014 National Automobile Dealers Association (NADA) Annual Report.

Depreciation and insurance are not included in the operating cost.
By 2030, the average passenger-car vehicle operating cost is estimated to total $\$ 0.209$ per mile. Operating costs for future years were developed based on: 1) forecasts of future crude oil prices, 2) assumed improvements in the average gas mileage associated with government-mandated fuel efficiency standards, and 3 ) inflation adjusted costs of tires and maintenance.

Commercial vehicle operating costs are much more difficult to calculate due to the variation in truck sizes and types and availability of corporate fleet information. For this study, the vehicle operating costs for commercial vehicle trips was assumed to be three times greater than passenger cars in 2015. This differential increases over time since anticipated fuel efficiency improvements for passenger vehicles are not assumed for commercial vehicles. By 2030, it was assumed that commercial vehicle operating costs were four times greater than passenger cars on a per mile basis.

### 6.2.5 Annual Inflation Rates

Toll rates, VOT and components of VOC are increased annually in the modeling process assuming they keep pace with inflation. Average annual inflation rates are shown in Table 6.3 for various historical and forecast time periods. All inflation rates are for the Charlotte-Concord-Gastonia, NC-SC Metropolitan Statistical Area (Charlotte MSA), represented in Figure 6.3. Historically, the annual rate of inflation has averaged 2.2 percent from 1995 to 2000, 1.8 percent from 2000 to 2005 and 2.6 percent from 2005 to 2010. The average inflation rate decreased to 1.6 percent from 2010 to 2015, due in part to steeply decreasing gasoline and oil prices.

Annual forecasted CPI indices were obtained from Moody's Analytics for 2016 through 2045. As shown in Table 6.3, annual inflation is forecast to average 2.5 percent between 2015 and 2019, 2.3 percent between 2019 and 2025, and 2.1 percent after 2025. Annual inflation is forecast to average 2.2 percent
per year from 2015 to 2040, which equals the historical inflation rate of the past 25 years, from 1990 to 2015.

Table 6.3
Actual and Estimated Average Annual Inflation Rates ${ }^{(1)}$

| Historical Time Period (2) | AAPC (4) |
| :--- | :---: |
| 1995-2000 | $2.2 \%$ |
| $2000-2005$ | $1.8 \%$ |
| $2005-2010$ | $2.6 \%$ |
| 2010-2015 | $1.6 \%$ |
| Forecast Time Period (3) |  |
| 2015-2019 | $2.5 \%$ |
| 2019-2025 |  |
| 2025-2030 | $2.3 \%$ |
| 2030-2040 |  |
| 1) Historical data and forecasts are for the |  |
| Charlotte-Concord-Gastonia, |  |
| NC-SC Metropolitan Statistical Area. |  |
| 2) Based on Bureau of Labor Statistics CPI |  |
| (Index 1982-84 = 100) data for all |  |
| Urban Consumers through the third |  |
| quarter of 2015. |  |
| 3) Based on annual forecasts developed by |  |
| Moody's Analytics, last updated . |  |
| on 1/22/2016. |  |
| 4) AAPC is an abbreviation for Average |  |
| Annual Percent Change. |  |



### 6.3 Basic Assumptions

The traffic and revenue estimates for the Monroe Expressway are predicated on the following basic assumptions, which are considered reasonable for purposes of the base case forecast:

1. The Monroe Expressway, in its entirety as described in Section 3.2 Project Configuration, will open to traffic on January 1, 2019.
2. The configuration, number of lanes, and number of access points on the Monroe Expressway will not change during the forecast period.
3. Tolls will be charged for the three vehicle classes and two payment types described in Section 6.2.1 Toll Structure. The toll rates will be increased annually from the initial 2019 toll rates, to keep up with inflation. The toll rates and tolling zone locations are provided in Section 6.6 Recommended Toll Rates.
4. No transportation improvement projects, particularly new roads, additional road capacity, or new interchanges on limited access roads will be constructed during the forecast period, other than those included in the MRM15v1.1. Assumed roadway improvements are discussed in Section 6.4.
5. The annual percentage of ETC and BBM transactions will match the assumed market shares described in Section 6.2.2.
6. Economic growth in the project study area by TAZs will occur as forecast by the independent economist.
7. Revenue leakage due to unreadable or uncollectible ETC or BBM transactions, or any transactions that cannot be processed and payment collected, will occur. The adjustments made to gross toll revenue forecasts, to account for uncollectible toll revenue, are based on actual experience on the Triangle Expressway. If actual experience on the Monroe Expressway differs from the Triangle Expressway with regard to adjustments for uncollectible toll revenue, the resulting net toll revenue forecasts for Monroe Expressway will differ from those forecast in this study.
8. The leakage estimates contained in this report are dependent upon the selection of appropriate toll collection technology and the adoption of business rules and enforcement procedures designed to minimize the loss of revenue.
9. Annual inflation rates will average those presented in Section 6.2.5.
10. The Monroe Expressway will be well maintained, efficiently operated, effectively signed, and promoted to encourage maximum usage and to reach the assumed percentage goals for ETC usage.
11. Motor fuel will remain in adequate supply throughout the forecast period. Fuel price forecasts were obtained from the U.S. Energy Information Administration in a report titled Annual Energy Outlook 2015 with projections to 2040. The forecast fuel costs were incorporated into the estimated vehicle operating costs.
12. No national or regional emergency will arise that would abnormally restrict the use of motor vehicles.

Any significant departure from these basic assumptions could materially affect traffic and revenue potential on the Monroe Expressway.

### 6.4 Future Transportation Improvements

CDM Smith identified the roadway improvements assumed in the MRM15v1.1 network years 2015, 2025, 2030 and 2040. The assumed improvements in the MRM were compared against those listed in the State Transportation Improvement Program (STIP), and various planning documents in the MRM area obtained from Metropolitan Planning Organizations (MPOs) and Rural Planning Organizations (RPOs). The goal was two-fold:

1. To verify that roadway improvements listed in current transportation plans and long range plans are present in the MRM in the appropriate years, and
2. To identify roadway improvements that may substantially impact the travel demand on the Monroe Expressway.

The following are the documents that were reviewed in order to identify planned roadway improvements in the MRM model area.

1. NCDOT Current STIP, November 2015
2. Cabarrus / Rowan Urban Area Metropolitan Planning Organization, Draft 2040 Metropolitan Transportation Plan, March 2014
3. CRTPO 2040 Metropolitan Transportation Plan Air Quality Conformity Determination Report, March 2014
4. The Way Forward: 2040 Metropolitan Transportation Plan, Gaston-Cleveland-Lincoln Metropolitan Planning Organization, Adopted March 27, 2014
5. Rock Hill - Fort Mill Area Transportation Study FY 2014-19 TIP Financial Statement
6. North Carolina Department of Transportation, Prioritization 4.0, NCDOT Strategic Prioritization Office of Transportation, July 2015

CDM Smith verified that the planned roadway improvements listed in the above documents are reflected in the MRM networks. Based on comments from the NCDOT, the assumed project completion date was changed from that assumed in the MRM network for several projects, and the proper adjustment was made to the networks. CDM Smith also verified that there were no network improvements assumed in the MRM networks that were not listed in a planning document, were located in the study area, and were likely to have an impact on Monroe Expressway usage. In the modeling assignments, CDM Smith used the same roadway assumptions that were included in the MRM15v1.1 with a few changes to assumed completion dates.

Table 6.4 and Figure 6.4 present a list of selected future roadway improvement projects that are reflected in the MRM networks, are identified in future roadway plans, and are in the vicinity of the Monroe Expressway. These projects were selected because they were fairly major in scope, or were located in the study corridor.

Notable Mecklenburg County roadway improvements in the vicinity of the proposed Monroe Expressway include the addition of express lanes on I-77, I-485 and US 74. The construction of additional toll-free capacity is planned for segments of I-485, US 74 and NC 51. Each of these projects will have some impact on traffic volumes and travel patterns in the area.

Table 6.4
Selected Future-Year Roadway Improvements
Assumed in the Current Monroe Expressway Traffic and Revenue Study

| Project <br> STIP ID | Roadway | Location | Description | Assumed <br> Opening <br> Date |
| :---: | :---: | :---: | :---: | :---: |
| Mecklenburg County |  |  |  |  |
| U-0209 | US 74 | Brookshire Freeway to Idlewild Road in Charlotte | Widen to 4 lanes per direction from NC 27 to Wallace Lane and construct interchanges with Sharon Amity Road and Idlewild Road | Dec. 2016 |
| R-0211EC | I-485 | Weddington Road overpass | Construct new interchange between I-485 and Weddington Road | Jan. 1, 2019 |
| U-2509 | US 74 | Charlotte Outer Loop to Conference Drive in Charlotte | Widen to 3 general purpose lanes per direction and build express lanes in median | Jan. 1, 2025 |
| U-4713A | New Road | SR 3448 (Pleasant Plains Road) to SR 1009 (E. John Street) | Add 1 lane per direction on new location |  |
| U-4713B | New Road | SR 1009 (John Street) to SR 3457 (Campus Ridge Road) | Add 1 lane per direction on new location |  |
| U-5007 | NC 51 | Matthews Township Parkway to SR 3128 (Lawyers Road) | Add 1 general purpose lane per direction | Jan. 1, 2025 |
| 1-5507 | I-485 | I-77 South of Charlotte to US 74 (Independence Boulevard) | Add 1 express lane per direction | Jan. 1, 2019 |
| U-5526 | US 74 | I-277 to Wallace Lane | Convert existing and proposed busway to reversible express lane from I-277 to Albemarle Road and add 1 express lane per direction from Albemarle Road to Wallace Lane | Jan. 1, 2018 |
| 1-5718 | I-77 | Phase A: I-485 (Exit 1) to Woodlawn Road (Exit 6) <br> Phase B: Woodlawn Road (Exit 6) to I-277/US 74 (Exit 9) <br> Phase C: I-277/US 74 (Exit 9) to I-277/NC 16 (Exit 11) <br> Phase D: I-277/US 74/NC 27 Interchange <br> Phase E: I-277/NC 16, US 21 Interchange | Add 1 express lane per direction Add 1 express lane per direction Add 1 express lane per direction Interchange improvements Interchange improvements | Jan. 1, 2030 |
| U-5763 | NC 51 | SR 3356 (Sardis Road) to SR 1009 (Monroe Road) | Add 1 general purpose lane per direction | Jan. 1, 2020 |
| U-5804 | SR 3448 | Fullwood Lane to Weddington Road | Add 1 lane per direction | Jan. 1, 2017 |
| U-5805 | SR 1009 | Intersection with Idlewild Road | Construct improvements | Jan. 1, 2021 |
| Union County |  |  |  |  |


| U-2549 | Monroe Northern Loop | US 74 TO SR 1751 (Walkup Avenue) at SR 1763 (Bivens Road) | Add 2 lanes per direction on new location | Jan. 1, 2030 |
| :---: | :---: | :---: | :---: | :---: |
| U-3467 | New Road | NC 16 (Providence Road) to SR 84 (Weddington Road) | Extend SR 1316 (Rea Road) from NC 16 (Providence Road) to SR 84 (Weddington Road) | Jan. 1, 2020 |
| U-4714 | SR 1009 | SR 3448 (Trade Street) to SR 1377 (Wesley Chapel-Stouts Road) | Add 1 general purpose lane per direction and convert to superstreet | Jan. 1, 2025 |
| U-4913 | Idlewild Rd | I-485 TO SR 1524 (Stevens Mill Road) | Add 1 general purpose lane per direction | Jan. 1, 2022 |
| W-5520 | US 74 | Indian Trail Fairview Road to Wesley Chapel Stouts Road | Convert existing full movement signalized intersections to signalized superstreet design | Jan. 1, 2017 |
| U-5703 | US 74 | SR 1514 (Rocky River Road) intersection | Reconfigure to superstreet design | Jan. 1, 2022 |
| U-5723 | US 74 | US 601 Interchange | Construct improvements | Jan. 1, 2022 |
| U-5764 | US 74 | Hanover Drive to SR 1007 (Rocky River Road) | Add 1 general purpose lane per direction | Jan. 1, 2023 |



Roadway improvements within Union County that will directly impact travel demand on the proposed Monroe Expressway include:

1. U-5764 - the addition of 1 general purpose lane in each travel direction on US 74 between Hanover Drive and SR 1007. This roadway improvement is anticipated to be completed by January 1, 2023.
2. U-2549 - the construction of the Monroe Northern Loop, a new 4-lane roadway that will provide enhanced route options for travel in the area of Monroe. This roadway improvement is assumed to be completed by January 1, 2030.
3. U-4714 - the addition of 1 general purpose lane in each travel direction on SR 1009 (Old Charlotte Highway) between SR 3448 (Trade Street) to SR 1377 (Wesley Chapel-Stouts Road). This improvement is assumed to be completed by January 1, 2025.

These three roadway improvements had relatively small, system wide negative impacts on Monroe Expressway toll transactions based on the assignment process.

### 6.5 Toll Rate Sensitivity Analysis

A toll rate sensitivity analysis was conducted at 2019 opening year conditions, to identify the optimum per-mile toll for the Monroe Expressway. The analysis was conducted by running a series of assignments with gradually increasing per-mile toll rates in order to identify a point on the curve that optimizes toll revenue, but still leaves some room for upward adjustment.

Figure 6.5 illustrates the 2019 Class 1 (two-axle vehicle) toll sensitivity curve for ETC assuming the entire toll road is operational. The $x$-axis represents the range of tested per-mile toll rates. The y-axis represents the resulting average weekday toll revenue for all three toll classes and both methods of payment.

As shown in the figure, the selected, optimal ETC toll rate for a Class 1 vehicle traveling the full length of the toll road is approximately $\$ 0.14$ per mile. The associated Class 1 BBM toll rate for the same trip is about $\$ 0.22$ per mile (assuming the current ETC-BBM toll relationship). Actual tolls collected for each toll zone will be equal to the length of that mainline section multiplied by the per-mile toll rate, and rounded up to the nearest penny. The selected toll rate is set slightly below the rate which would maximize toll revenue in order to provide a limited "margin of safety" for setting future rates. Rates are assumed to increase annually to keep pace with inflation.

### 6.6 Recommended Toll Rates by Toll Zone

Table 6.5 shows Class 1 ETC and BBM rates, by tolling zone, for all years from 2019 through 2040. These toll rates are based on the 2019 selected optimum toll rates of $\$ 0.14$ per mile for Class 1 ETC and $\$ 0.22$ per mile for Class 1 BBM. In all years, Class 2 rates are double Class 1 rates, and Class 3 rates are four times Class 1 rates. ETC toll rates receive a 35 percent discount from BBM toll rates. As shown in this table, annual rate adjustments take place each January 1 to take into account the impacts of annual inflation (see Table 6.3 for a description of the assumed inflation rates).

Figure 6.6 graphically displays the ETC toll rates in 2019 and 2030 at each tolling zone location for Class 1, Class 2 and Class 3 vehicles. The opening-year ETC toll for a full-length trip through all seven tolling zones on the Monroe Expressway will be $\$ 2.54$ for Class 1 vehicles, increasing to $\$ 3.22$ in 2030 . A

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[^1]Not To Scale

large truck (Class 3) with ETC will pay $\$ 10.16$ in 2019, and $\$ 12.88$ in 2030, to travel the length of the Monroe Expressway.

Figure 6.7 illustrates the BBM toll rates for Class 1, Class 2, and Class 3 vehicles by location for 2019 and 2030. In 2019, a Class 1 vehicle with BBM will pay $\$ 3.92$ for a full-length trip, increasing to $\$ 4.99$ in 2030. A Class 3 vehicle will pay $\$ 15.68$ in 2019 and $\$ 19.96$ in 2030 for the same trip.

### 6.7 Estimated Weekday Traffic Volumes

Estimates of weekday mainline traffic volumes in 2019, 2025, 2030 and 2040 are shown in Figure 6.8. Vehicles will be tolled on each mainline section via overhead gantries. As can be seen in the figure, weekday mainline traffic volumes increase steadily from the easternmost mainline section to the westernmost section. The traffic volumes shown in the figure do not include any downward "ramp-up" adjustment, which is incorporated in the early years of the annual traffic forecasts. Total 2019 weekday traffic volumes range from 9,100 on the easternmost section (Zone 1: US 74 to Austin Chaney Rd.) to a high of 33,100 on the westernmost section (Zone 7: Indian Trail Fairview Rd. to US 74).

In 2030, weekday mainline traffic volumes on the Monroe Expressway are forecast to range from 11,200 (easternmost section) to 42,800 on the westernmost section. Weekday traffic volumes in 2040 are forecast to range from 12,300 on the easternmost section to 48,200 on the westernmost section.

Forecast traffic volumes decrease from 2025 to 2030 in Zone 2 (Austin Chaney Rd - NC 200). This decrease is due to a roadway improvement (Project STIP U-2549) that is assumed to open on January 1, 2030. The improvement consists of a new four-lane road, called the Monroe Northern Loop, that extends from US 74 to SR 1751 (Walkup Ave.) at Bivens Rd. The project is shown in Figure 6.4. This project is estimated to draw traffic off of certain sections of the Monroe Expressway, most notably from Toll Zone 2. On a systemwide basis, the negative impacts of the Monroe Northern Loop are relatively small.

### 6.8 Sample Travel Time and Distance Savings

Figure 6.9 illustrates estimated average travel-time savings during the three-hour AM Peak Period (6:30 through 9:30 AM), in the westbound direction, in 2019 and 2030, by comparing travel time on the Monroe Expressway to the best alternative toll-free route. Estimated travel times were obtained from the calibrated MRM assignments. The travel times represent average minutes of travel during the threehour peak period. Actual travel times will vary from these estimates, sometimes significantly, based on actual roadway and weather conditions, and on actual travel demand in shorter increments of time.

The following three trips are shown in Figure 6.9 to illustrate potential travel time savings by using the Monroe Expressway:

- Trip 1 - from Marshville to Mathews,
- Trip 2 - from Monroe to Mathews, and
- Trip 3 from Indian Trail to Mathews.

Not To Scale


## Not To Scale



ESTIMATED 2019, 2025, 2030 AND 2040


ESTIMATED AVERAGE TRAVEL TIME SAVINGS AM PEAK PERIOD - WESTBOUND FIGURE 6.9

The trip comparison data includes the distance traveled, the average travel time in minutes, and the passenger-car ETC toll for the trip on the Expressway. For example, in 2019, a trip from Marshville to Mathews (Trip 1) on the Monroe Expressway would save about 0.6 miles and an average of 17 minutes compared to the same trip via US 74 during the AM Peak Period. In 2030, the average-time savings for using the Monroe Expressway is estimated to increase to 23 minutes. The toll cost for a passenger car making this trip via the Monroe Expressway would be $\$ 2.54$ in 2019 and $\$ 3.22$ in 2030 . The cost of the toll is accounted for in the route choice assigned in the modeling process.

A trip from Monroe to Mathews (Trip 2) on the Monroe Expressway compared to US 74 would include an increased distance of 2.6 miles, and an average travel-times savings of 8 minutes in 2019 and 12 minutes in 2030. The toll cost for a passenger car making this trip via the Monroe Expressway would be \$1.31 in 2019 and \$1.66 in 2030.

A trip from Indian Trail to Mathews (Trip 3) on the Monroe Expressway compared to US 74 would include an increased distance of 0.5 miles, and an average travel-times savings of 5 minutes in 2019 and 6 minutes in 2030. The toll cost for a passenger car making this trip via the Monroe Expressway would be $\$ 0.57$ in 2019 and $\$ 0.72$ in 2030.

### 6.9 Assignment-Year Annualization and Ramp-Up Adjustments

This section describes the methodology for developing the following forecasts: 1) the average weekday toll revenue, 2) the calendar-year toll transactions and toll revenue, 3) the ramp-up adjustment for the opening year, and 4) the conversion of traffic and toll revenue to a fiscal year. This process was performed for the assignment years 2019, 2025, 2030 and 2040. Table 6.6 illustrates the process for year 2019 .

### 6.9.1 Calendar Year 2019 Average Weekday Toll Transactions and Toll Revenue

As seen in Table 6.6, Class 1 toll transactions for each toll zone and by ETC and BBM, are multiplied by their corresponding toll rates to arrive at the average weekday toll revenue. Similarly, the combined Class 2 and 3 toll transactions, are multiplied by an average weighted toll for the two classes to arrive at the average weekday toll revenue for each tolling zone. The result in 2019 is an estimated 145,888 average weekday transactions and $\$ 72,686$ average weekday toll revenue.

### 6.9.2 Calendar Year 2019 Annual Toll Transactions and Toll Revenue

The average weekday toll transactions and toll revenue are converted into an annual year forecast. This annualization is based on the assumption that there will be the equivalent of 327.3 weekdays in a calendar year for the Monroe Expressway study. This conversion takes into account lower estimated traffic volumes on weekend days and holidays. For this study, it was assumed that average weekend day traffic will be 68 percent of average weekday traffic on the Expressway. As shown in Table 6.6, the annualized toll transactions for calendar year 2019 will be 47,745,000 (145,888 average weekday transactions multiplied by 327.3). Similarly, annualized toll revenue for calendar year 2019 will be $\$ 23,789,000$ (\$72,686 average weekday revenue multiplied by 327.3).

Table 6.6 Development of FY 2019 Toll Transactions and Gross Toll Revenue Estimates
I. Calendar Year - Estimated Average Weekday Toll Transactions

|  |  | Calendar Year 2019 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Class 1 |  | Classes 2 and 3 |  | All Vehicles |
| Toll Zone Number and Name |  | BBM | ETC | BBM | ETC |  |
| 1 | US 74 - Austin Chaney Rd. | 3,252 | 4,694 | 350 | 823 | 9,119 |
| 2 | Aystin Chaney Rd. - NC 200 | 4,300 | 6,365 | 397 | 959 | 12,021 |
| 3 | NC 200 - US 601 | 6,121 | 8,876 | 542 | 1,286 | 16,825 |
| 4 | US 601 - N. Rocky River Rd. | 7,823 | 11,561 | 659 | 1,564 | 21,607 |
| 5 | N. Rocky River Rd. - Union Indian Trail Rd. | 9,019 | 13,196 | 757 | 1,782 | 24,754 |
| 6 | Union Indian Trail Rd. - Indian Trail/Fairview Rd. | 10,619 | 15,216 | 790 | 1,833 | 28,458 |
| 7 | Indian Trail/Fairview Rd. - US 74 | 12,733 | 17,624 | 844 | 1,902 | 33,103 |
|  | Total | 53,867 | 77,533 | 4,338 | 10,150 | 145,888 |

II. Calendar Year - Toll Rates

|  |  | Calendar Year 2019 |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Class 1 |  |  |  | Average Weighted Toll for Classes 2 and 3 |  |  |  |
| Toll Zone Number and Name |  | BBM |  | ETC |  | BBM |  | ETC |  |
| 1 | US 74-Austin Chaney Rd. | \$ | 0.65 | \$ | 0.42 | \$ | 2.16 | \$ | 1.40 |
| 2 | Aystin Chaney Rd. - NC 200 |  | 0.86 |  | 0.56 |  | 2.86 |  | 1.86 |
| 3 | NC 200 - US 601 |  | 0.39 |  | 0.25 |  | 1.30 |  | 0.83 |
| 4 | US 601 - N. Rocky River Rd. |  | 0.85 |  | 0.55 |  | 2.83 |  | 1.83 |
| 5 | N. Rocky River Rd. - Union Indian Trail Rd. |  | 0.29 |  | 0.19 |  | 0.97 |  | 0.63 |
| 6 | Union Indian Trail Rd. - Indian Trail/Fairview Rd. |  | 0.48 |  | 0.31 |  | 1.60 |  | 1.03 |
| 7 | Indian Trail/Fairview Rd. - US 74 |  | 0.40 |  | 0.26 |  | 1.33 |  | 0.87 |

III. Calendar Year - Estimated Average Weekday Gross Toll Revenue

| Toll Zone Number and Name |  | Calendar Year 2019 |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Class 1 |  |  |  | Classes 2 and 3 |  |  |  | All Vehicles |  |
|  |  | BBM |  | ETC |  | BBM |  | ETC |  |  |  |
| 1 | US 74 - Austin Chaney Rd. | \$ | 2,114 | \$ | 1,972 | \$ | 755 | \$ | 1,152 | \$ | 5,993 |
| 2 | Aystin Chaney Rd. - NC 200 |  | 3,698 |  | 3,564 |  | 1,136 |  | 1,783 |  | 10,182 |
| 3 | NC 200 - US 601 |  | 2,387 |  | 2,219 |  | 705 |  | 1,067 |  | 6,378 |
| 4 | US 601 - N. Rocky River Rd. |  | 6,649 |  | 6,359 |  | 1,864 |  | 2,863 |  | 17,735 |
| 5 | N. Rocky River Rd. - Union Indian Trail Rd. |  | 2,616 |  | 2,507 |  | 734 |  | 1,123 |  | 6,980 |
| 6 | Union Indian Trail Rd. - Indian Trail/Fairview Rd. |  | 5,097 |  | 4,717 |  | 1,264 |  | 1,888 |  | 12,966 |
| 7 | Indian Trail/Fairview Rd. - US 74 |  | 5,093 |  | 4,582 |  | 1,122 |  | 1,655 |  | 12,453 |
|  | Total |  | 27,654 |  | 25,920 | \$ | 7,580 |  | 11,532 | \$ | 72,686 |


| Annualizat <br> Period | Annualization Procedure (in thousands) <br> Annualization Factor: 327.3 days per year | Annual Gross <br> Toll Revenue |
| :---: | :---: | :---: |
|  | Annual Toll Transactions |  |
| Calendar Year (CY) 2019 | 47,745 | \$23,789 |
| Apply Ramp-Up Factor To CY 2019 | 0.60 | 0.60 |
| Dampened CY 2019 | 28,647 | \$14,273 |
| Conversion to Fiscal Year 2019 |  |  |
| Half of CY 2019 (January-June) | 14,324 | \$7,137 |

### 6.9.3 Calendar Year 2019 Ramp-Up Adjustment

The annualized transactions and toll revenue were dampened to reflect "ramp-up" in 2019. With new toll facilities, it often takes time for motorists to learn about the new road and change their travel patterns, particularly for motorists who may use the road infrequently or live far from the road. It also accounts for the time it takes motorists to learn about and feel comfortable with the toll collection methods, particularly if there aren't existing toll roads in the area. The duration and level of ramp-up adjustments can be directly affected by a well-conceived promotion and signing program.

For the purposes of this study, a 36-month ramp-up period was assumed. The traffic and toll revenue forecasts for the first three years of operation was adjusted downward to reflect the time it will take to gradually build up to full demand. Table 6.7 presents the ramp-up factors assumed for this study.

Table 6.7
Assumed Ramp-Up Factors

| Calendar <br> Year | Ramp-Up <br> Factor (1) |
| :---: | :---: |
| 2019 | 0.600 |
| 2020 | 0.800 |
| 2021 | 0.950 |
| 1) Applied to calendar year |  |
| transaction estimates. |  |

After applying ramp-up to 2019 estimates, as seen in Table 6.6, the dampened annual toll transactions total 28.6 million, and the dampened toll revenue totals $\$ 14.3$ million.

### 6.9.4 Conversion of 2019 to a Fiscal Year

Transaction and toll revenue forecasts on a calendar year basis were divided in half and allocated to the appropriate fiscal year, which is assumed to run from July 1 of one calendar year to June 30 of the following calendar year. Because the Monroe Expressway is assumed to open on January 1, 2019, it will only be open for half of fiscal year 2019 (July 1, 2018 - June 30, 2019). As seen in Table 6.6, FY 2019 transactions and toll revenue are forecast to total 14.3 million and $\$ 7.1$ million respectively.

### 6.9.5 FY 2025, FY 2030 and FY 2040 Transactions and Toll Revenue

The same procedure as described in the previous sections for FY 2019 was followed to develop the fiscal year toll transactions and toll revenue. Tables 6.8 and 6.9 illustrate the procedures to develop FY 2030 and FY 2040. In each of these instances, two adjacent calendar years are calculated, and re-allocated to form the fiscal year estimates. For example, FY 2030 is composed of half of calendar year 2029 and half of calendar year 2030. As mentioned previously, model years are 2019, 2025, 2030, and 2040.
Intermediate years were developed by interpolating between assignment years. Ramp-up adjustments were not made to any assignment years except 2019. The same annualization assumptions were made for all assignment years.
Table 6.8
Development of FY $\mathbf{2 0 3 0}$ Toll Transactions and Gross Toll Revenue Estimates


| 1 | US 74-Austin Chaney Rd. |
| :--- | :---: |
| 2 | Austin Chaney Rd. - NC 200 |
| 3 | NC 200 - US 601 |
| 4 | US 601 - N. Rocky River Rd. |
| 5 | N. Rocky River Rd. - Unionville Indian Trail Rd |
| 6 | Inionville Indian Trail Rd. - Indian Trail Fairvie |
| 7 | Indian Trail Fairview Rd. - US 74 |
|  | Total |

\$39,945 \$45,182
$\qquad$





Table 6.9
Development of FY 2040 Toll Transactions and Gross Toll Revenue Estimates




I. Calendar Year - Estimated Average Weekday Toll Transactions


|  |  | Calendar Year 2039 |  |  |  |  |  |  | Calendar Year 2040 |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Class 1 |  |  | Average Weighted Toll for Classes 2 and 3 |  |  |  | Class 1 |  |  |  | Average Weighted Toll for Classes 2 and 3 |  |  |  |
|  | Toll Zone Number and Name | BBM | ETC |  | BBM |  | ETC |  | BBM |  | ETC |  | BBM |  | ETC |  |
| 1 | US 74 - Austin Chaney Rd. | 1.00 | \$ | 0.64 | \$ | 3.31 | \$ | 2.14 | \$ | 1.02 | \$ | 0.66 | \$ | 3.38 | \$ | 2.19 |
| 2 | Austin Chaney Rd. - NC 200 | 1.32 |  | 0.86 |  | 4.38 |  | 2.85 |  | 1.35 |  | 0.88 |  | 4.48 |  | 2.91 |
| 3 | NC 200 - US 601 | 0.60 |  | 0.38 |  | 1.99 |  | 1.27 |  | 0.61 |  | 0.39 |  | 2.04 |  | 1.30 |
| 4 | US 601 - N. Rocky River Rd. | 1.30 |  | 0.84 |  | 4.33 |  | 2.80 |  | 1.33 |  | 0.86 |  | 4.43 |  | 2.86 |
| 5 | N. Rocky River Rd. - Unionville Indian Trail Rd. | 0.44 |  | 0.29 |  | 1.49 |  | 0.96 |  | 0.45 |  | 0.30 |  | 1.52 |  | 0.99 |
| 6 | Inionville Indian Trail Rd. - Indian Trail Fairview Rd. | 0.74 |  | 0.47 |  | 2.45 |  | 1.58 |  | 0.75 |  | 0.49 |  | 2.50 |  | 1.61 |
| 7 | Indian Trail Fairview Rd. - US 74 | 0.61 |  | 0.40 |  | 2.04 |  | 1.33 |  | 0.63 |  | 0.41 |  | 2.08 |  | 1.36 |

Total

### 6.10 Fiscal Year Gross Toll Transactions and Toll Revenue

This section presents annual toll transactions and annual gross toll revenue forecasts from FY 2019 through FY 2058. Gross toll revenue consists of toll revenue from all toll transactions, prior to accounting for leakage or processing fee revenue.

Estimated annual toll transactions by Class 1 and combined Class 2 and 3 vehicles is presented in Table 6.10 and Figure 6.10. Annual transactions are expected to increase from 14.3 million in FY 2019, to 57.3 million in FY 2025, 61.5 million in FY 2030, and 68.8 million in FY 2040. Traffic estimates for FY 2019 through FY 2022 were adjusted downward to reflect the three-year ramp-up period as discussed in Section 6.8.3.

Table 6.10
Estimated Annual Toll Transactions for Monroe Expressway (in thousands)

| Fiscal <br> Year (1) | Class 1 |  |  | Classes 2 and 3 |  |  | All Vehicles |  |  | $\begin{gathered} \text { Percent } \\ \text { ETC } \\ \hline \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | BBM | ETC | Total | BBM | ETC | Total | BBM | ETC | Total |  |
| 2019 (2,3) | 5,289 | 7,612 | 12,901 | 426 | 997 | 1,422 | 5,715 | 8,609 | 14,324 | 60.1 |
| 2020 (3) | 12,521 | 18,203 | 30,724 | 1,002 | 2,355 | 3,358 | 13,523 | 20,559 | 34,082 | 60.3 |
| 2021 (3) | 16,040 | 23,714 | 39,754 | 1,271 | 3,009 | 4,280 | 17,311 | 26,723 | 44,034 | 60.7 |
| 2022 (3) | 18,317 | 27,537 | 45,854 | 1,436 | 3,427 | 4,863 | 19,753 | 30,965 | 50,717 | 61.1 |
| 2023 | 19,261 | 29,455 | 48,716 | 1,494 | 3,594 | 5,088 | 20,755 | 33,049 | 53,804 | 61.4 |
| 2024 | 19,754 | 30,735 | 50,489 | 1,516 | 3,676 | 5,192 | 21,270 | 34,411 | 55,682 | 61.8 |
| 2025 | 20,124 | 31,853 | 51,977 | 1,531 | 3,743 | 5,274 | 21,655 | 35,596 | 57,251 | 62.2 |
| 2026 | 20,253 | 32,676 | 52,929 | 1,538 | 3,794 | 5,332 | 21,791 | 36,470 | 58,261 | 62.6 |
| 2027 | 20,274 | 33,403 | 53,677 | 1,543 | 3,845 | 5,388 | 21,817 | 37,248 | 59,065 | 63.1 |
| 2028 | 20,295 | 34,148 | 54,443 | 1,549 | 3,897 | 5,446 | 21,844 | 38,045 | 59,889 | 63.5 |
| 2029 | 20,317 | 34,911 | 55,228 | 1,554 | 3,950 | 5,504 | 21,871 | 38,861 | 60,732 | 64.0 |
| 2030 | 20,304 | 35,628 | 55,932 | 1,563 | 4,012 | 5,575 | 21,867 | 39,640 | 61,507 | 64.4 |
| 2031 | 20,271 | 36,262 | 56,534 | 1,575 | 4,084 | 5,660 | 21,847 | 40,347 | 62,193 | 64.9 |
| 2032 | 20,255 | 36,874 | 57,129 | 1,588 | 4,160 | 5,748 | 21,844 | 41,034 | 62,878 | 65.3 |
| 2033 | 20,239 | 37,497 | 57,735 | 1,602 | 4,237 | 5,839 | 21,841 | 41,733 | 63,574 | 65.6 |
| 2034 | 20,223 | 38,129 | 58,352 | 1,615 | 4,315 | 5,931 | 21,838 | 42,445 | 64,283 | 66.0 |
| 2035 | 20,206 | 38,773 | 58,979 | 1,629 | 4,395 | 6,024 | 21,835 | 43,168 | 65,003 | 66.4 |
| 2036 | 20,190 | 39,427 | 59,618 | 1,643 | 4,476 | 6,119 | 21,833 | 43,904 | 65,737 | 66.8 |
| 2037 | 20,174 | 40,093 | 60,267 | 1,656 | 4,559 | 6,216 | 21,831 | 44,652 | 66,483 | 67.2 |
| 2038 | 20,158 | 40,770 | 60,928 | 1,670 | 4,644 | 6,314 | 21,828 | 45,413 | 67,242 | 67.5 |
| 2039 | 20,142 | 41,458 | 61,600 | 1,684 | 4,730 | 6,414 | 21,826 | 46,187 | 68,014 | 67.9 |
| 2040 | 20,126 | 42,158 | 62,283 | 1,699 | 4,817 | 6,516 | 21,824 | 46,975 | 68,799 | 68.3 |
| 2041 | 20,090 | 42,877 | 62,967 | 1,709 | 4,895 | 6,603 | 21,799 | 47,772 | 69,570 | 68.7 |
| 2042 | 20,035 | 43,616 | 63,652 | 1,714 | 4,961 | 6,676 | 21,750 | 48,578 | 70,327 | 69.1 |
| 2043 | 19,981 | 44,369 | 64,349 | 1,720 | 5,029 | 6,749 | 21,701 | 49,398 | 71,098 | 69.5 |
| 2044 | 19,926 | 45,134 | 65,060 | 1,725 | 5,098 | 6,823 | 21,652 | 50,231 | 71,883 | 69.9 |
| 2045 | 19,872 | 45,912 | 65,784 | 1,731 | 5,167 | 6,899 | 21,603 | 51,079 | 72,683 | 70.3 |
| 2046 | 19,822 | 46,589 | 66,411 | 1,734 | 5,230 | 6,964 | 21,556 | 51,819 | 73,375 | 70.6 |
| 2047 | 19,777 | 47,161 | 66,938 | 1,734 | 5,286 | 7,019 | 21,510 | 52,447 | 73,957 | 70.9 |
| 2048 | 19,731 | 47,741 | 67,472 | 1,733 | 5,342 | 7,076 | 21,465 | 53,083 | 74,548 | 71.2 |
| 2049 | 19,686 | 48,327 | 68,013 | 1,733 | 5,399 | 7,132 | 21,419 | 53,726 | 75,146 | 71.5 |
| 2050 | 19,641 | 48,921 | 68,562 | 1,733 | 5,457 | 7,190 | 21,374 | 54,377 | 75,751 | 71.8 |
| 2051 | 19,589 | 49,519 | 69,108 | 1,732 | 5,515 | 7,247 | 21,321 | 55,034 | 76,355 | 72.1 |
| 2052 | 19,530 | 50,121 | 69,651 | 1,731 | 5,574 | 7,304 | 21,261 | 55,695 | 76,956 | 72.4 |
| 2053 | 19,471 | 50,731 | 70,202 | 1,730 | 5,633 | 7,363 | 21,201 | 56,364 | 77,564 | 72.7 |
| 2054 | 19,412 | 51,348 | 70,760 | 1,729 | 5,693 | 7,421 | 21,141 | 57,041 | 78,182 | 73.0 |
| 2055 | 19,353 | 51,973 | 71,327 | 1,727 | 5,753 | 7,481 | 21,081 | 57,727 | 78,807 | 73.3 |
| 2056 | 19,295 | 52,606 | 71,901 | 1,726 | 5,814 | 7,541 | 21,021 | 58,420 | 79,441 | 73.5 |
| 2057 | 19,236 | 53,246 | 72,482 | 1,725 | 5,876 | 7,601 | 20,962 | 59,122 | 80,084 | 73.8 |
| 2058 | 19,178 | 53,894 | 73,072 | 1,724 | 5,939 | 7,663 | 20,902 | 59,832 | 80,735 | 74.1 |
| 1) Fiscal year extends from July 1 through June 30 . <br> 2) Monroe Expressway assumed to open on January 1, 2019. Only 6 months of operation are assumed in FY 2019. <br> 3) Includes an assumed ramp-up to full traffic volumes. |  |  |  |  |  |  |  |  |  |  |




The ETC market share is anticipated to range from about 60 percent of total transactions in FY 2019 to about 68 percent in FY 2040. Table 6.11 shows the targeted ETC and BBM market shares, by calendar year, and the resulting ETC and BBM market shares from the model results. As shown in the table, the model market shares closely match with the targeted values.

Table 6.11
Annual Weekday ETC Market Share of Total Transactions
On the Monroe Expressway

| Calendar Year | Method-of-Payment Percent Market Share Targets |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Class 1 Vehicles |  |  | Class 2 and 3 Vehicles |  |  |
|  |  | ETC | BBM | Total | ETC | BBM | Total |
| 2019 | (1) | 59.0 | 41.0 | 100.0 | 70.0 | 30.0 | 100.0 |
| 2025 |  | 61.5 | 38.5 | 100.0 | 71.0 | 29.0 | 100.0 |
| 2030 |  | 64.0 | 36.0 | 100.0 | 72.0 | 28.0 | 100.0 |
| 2040 |  | 68.0 | 32.0 | 100.0 | 74.0 | 26.0 | 100.0 |
|  |  | Model Results - Percent Method of Payment Market Share |  |  |  |  |  |
| Calendar |  | Class 1 Vehicles |  |  | Class 2 and 3 Vehicles |  |  |
| Year |  | ETC | BBM | Total | ETC | BBM | Total |
| 2019 | (1) | 59.0 | 41.0 | 100.0 | 70.1 | 29.9 | 100.0 |
| 2025 |  | 61.3 | 38.7 | 100.0 | 71.0 | 29.0 | 100.0 |
| 2030 |  | 63.7 | 36.3 | 100.0 | 72.0 | 28.0 | 100.0 |
| 2040 |  | 67.7 | 32.3 | 100.0 | 73.9 | 26.1 | 100.0 |

1) Assumes the Monroe Expressway opens on January 1, 2019.

Transaction forecasts through 2040 were based on modeling results. Transactions after 2040 were based on extrapolating growth rates, taking into account prior year experience. Transactions between FY 2040 and FY 2058 were assumed to increase at the rates shown in Table 6.12.

Table 6.12
Outer Year Annual Transaction and Revenue
Growth Rate Assumptions: 2040-2058

| FY Period | Annual Growth Rate |  |
| :---: | :---: | :---: |
|  | Toll <br> Transactions | Toll Revenue |
| 2040-2045 | 1.1\% | 3.0\% |
| 2045-2058 | 0.8\% | 2.7\% |

ETC market share is forecast to increase from 2040 through 2058, ranging from an ETC market share of about 68 percent in 2040 to about 74 percent in 2040 . Throughout the forecast period, BBM transactions generally decline, as the proportion of ETC transactions is assumed to gradually increase.

Annual revenue estimates are provided in Table 6.13 and illustrated in Figure 6.9. Toll revenue estimates are provided for Class 1 and combined Class 2 and 3 vehicles, and by method of payment. The total annual gross toll revenue is expected to increase from about $\$ 7.1$ million in FY 2019 to about $\$ 82.7$ million in FY 2058. This reflects the impact of both traffic growth and annual toll increases. Toll revenue estimates for the first four fiscal years is dampened to reflect ramp-up. The assumed annual growth rates for gross toll revenue past the last modeling year of 2040 are shown in Table 6.12. Toll revenue growth was assumed to average 3.0 percent per year from 2040 to 2045, and 2.7 percent per year from 2045 to 2058.

ETC toll revenue is expected to account for about 53 percent of the total gross toll revenue in FY 2025, 60 percent in 2040, and 66 percent in 2058. In comparison, the ETC transactions are expected to comprise about 62 percent in FY 2025, 68 percent in FY 2040, and 74 percent in FY 2058. The lower ETC toll revenue percentages, compared to ETC transaction percentages, are due to the discounted toll that ETC users receive.

### 6.11 Fiscal Year Net Toll Revenue

This section describes the process by which gross toll revenue was adjusted to reflect unbillable and uncollectible BBM toll transactions. Net toll revenue consists of total collected toll revenue and processing fee revenue. Assumptions contained in this section are based on current NCTA business rules and recent actual experience on the Triangle Expressway.

### 6.11.1 Unbillable Bill by Mail Transaction Assumptions

Video tolling, while not new, contains inherent risks associated with various steps in the toll collection process. For example, it is possible that some plates may not be properly read, or vehicle owner address information with DMV records is incomplete. During calendar year 2015 NCTA was able to invoice 90.2 percent of Bill by Mail toll transactions on the Triangle Expressway. Approximately 6.7 percent of total Bill by Mail transactions were unbillable based on license plate images that could not be processed due to missing, blocked or damaged license plates, unreadable images, or other reasons. An additional 3.1 percent of Bill by Mail transactions were unbillable based on insufficient vehicle owner address information. Based on this information, 9.8 percent of forecast Monroe Expressway Bill by Mail transactions were assumed to be unbillable. In the development of net toll revenue estimates, expected toll revenue associated with unbillable Bill by Mail transactions was deducted from projected gross toll revenue.

### 6.11.2 Uncollectible Bill by Mail Revenue Assumptions

Under any video-tolling system, there is also an inherent collection risk of motorists who simply don't pay their invoices. Over the course of calendar years 2014 and 2015 NCTA was able to successfully collect 87.9 percent of invoiced Triangle Expressway Bill by Mail toll revenue. Based on this information 12.1 percent of billable Monroe Expressway Bill by Mail toll revenue was assumed to be uncollectible.

Table 6.13
Estimated Annual Gross Toll Revenue For Monroe Expressway (1) (in thousands \$)

| Fiscal | Class 1 |  |  | Classes 2 and 3 |  |  | All Vehicles |  |  | Percent ETC |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Year (2) | BBM | ETC | Total | BBM | ETC | Total | BBM | ETC | Total |  |
| $2019(3,4)$ | \$2,715 | \$2,545 | \$5,260 | \$744 | \$1,132 | \$1,876 | \$3,459 | \$3,677 | \$7,137 | 51.5 \% |
| 2020 (4) | 6,516 | 6,173 | 12,688 | 1,773 | 2,708 | 4,481 | 8,289 | 8,880 | 17,169 | 51.7 |
| 2021 (4) | 8,526 | 8,212 | 16,739 | 2,295 | 3,534 | 5,829 | 10,822 | 11,747 | 22,568 | 52.0 |
| 2022 (4) | 9,949 | 9,739 | 19,687 | 2,649 | 4,113 | 6,762 | 12,598 | 13,852 | 26,449 | 52.4 |
| 2023 | 10,717 | 10,630 | 21,347 | 2,818 | 4,407 | 7,225 | 13,535 | 15,037 | 28,572 | 52.6 |
| 2024 | 11,239 | 11,355 | 22,594 | 2,925 | 4,608 | 7,532 | 14,164 | 15,963 | 30,126 | 53.0 |
| 2025 | 11,701 | 12,103 | 23,804 | 3,021 | 4,800 | 7,821 | 14,722 | 16,903 | 31,625 | 53.4 |
| 2026 | 12,053 | 12,654 | 24,707 | 3,106 | 4,983 | 8,089 | 15,159 | 17,636 | 32,796 | 53.8 |
| 2027 | 12,351 | 13,201 | 25,552 | 3,188 | 5,163 | 8,351 | 15,539 | 18,364 | 33,903 | 54.2 |
| 2028 | 12,647 | 13,842 | 26,489 | 3,270 | 5,347 | 8,618 | 15,917 | 19,190 | 35,107 | 54.7 |
| 2029 | 12,930 | 14,464 | 27,394 | 3,358 | 5,544 | 8,902 | 16,288 | 20,008 | 36,296 | 55.1 |
| 2030 | 13,134 | 14,970 | 28,104 | 3,424 | 5,709 | 9,132 | 16,558 | 20,678 | 37,236 | 55.5 |
| 2031 | 13,322 | 15,467 | 28,790 | 3,494 | 5,890 | 9,384 | 16,816 | 21,357 | 38,173 | 55.9 |
| 2032 | 13,592 | 16,055 | 29,647 | 3,596 | 6,124 | 9,720 | 17,189 | 22,179 | 39,368 | 56.3 |
| 2033 | 13,861 | 16,668 | 30,529 | 3,703 | 6,360 | 10,062 | 17,563 | 23,028 | 40,591 | 56.7 |
| 2034 | 14,119 | 17,341 | 31,459 | 3,813 | 6,605 | 10,417 | 17,931 | 23,945 | 41,877 | 57.2 |
| 2035 | 14,411 | 18,039 | 32,450 | 3,926 | 6,866 | 10,792 | 18,337 | 24,905 | 43,242 | 57.6 |
| 2036 | 14,727 | 18,696 | 33,423 | 4,039 | 7,136 | 11,175 | 18,767 | 25,832 | 44,599 | 57.9 |
| 2037 | 15,043 | 19,390 | 34,433 | 4,156 | 7,420 | 11,576 | 19,199 | 26,809 | 46,009 | 58.3 |
| 2038 | 15,360 | 20,213 | 35,574 | 4,282 | 7,725 | 12,007 | 19,642 | 27,938 | 47,580 | 58.7 |
| 2039 | 15,653 | 20,930 | 36,583 | 4,409 | 8,025 | 12,434 | 20,061 | 28,956 | 49,017 | 59.1 |
| 2040 | 15,958 | 21,723 | 37,681 | 4,539 | 8,336 | 12,875 | 20,497 | 30,059 | 50,556 | 59.5 |
| 2041 | 16,269 | 22,577 | 38,846 | 4,661 | 8,653 | 13,313 | 20,930 | 31,230 | 52,159 | 59.9 |
| 2042 | 16,534 | 23,353 | 39,887 | 4,772 | 8,953 | 13,725 | 21,306 | 32,305 | 53,611 | 60.3 |
| 2043 | 16,806 | 24,229 | 41,034 | 4,884 | 9,259 | 14,143 | 21,690 | 33,488 | 55,177 | 60.7 |
| 2044 | 17,103 | 25,161 | 42,264 | 4,995 | 9,570 | 14,565 | 22,098 | 34,731 | 56,829 | 61.1 |
| 2045 | 17,398 | 26,152 | 43,551 | 5,113 | 9,885 | 14,998 | 22,511 | 36,038 | 58,549 | 61.6 |
| 2046 | 17,733 | 27,081 | 44,813 | 5,223 | 10,205 | 15,428 | 22,955 | 37,286 | 60,241 | 61.9 |
| 2047 | 18,049 | 27,965 | 46,014 | 5,325 | 10,519 | 15,843 | 23,373 | 38,484 | 61,857 | 62.2 |
| 2048 | 18,335 | 28,907 | 47,242 | 5,430 | 10,849 | 16,279 | 23,765 | 39,756 | 63,521 | 62.6 |
| 2049 | 18,668 | 29,869 | 48,537 | 5,537 | 11,191 | 16,728 | 24,204 | 41,061 | 65,265 | 62.9 |
| 2050 | 18,989 | 30,807 | 49,796 | 5,647 | 11,530 | 17,177 | 24,636 | 42,337 | 66,973 | 63.2 |
| 2051 | 19,292 | 31,762 | 51,054 | 5,759 | 11,884 | 17,643 | 25,051 | 43,646 | 68,697 | 63.5 |
| 2052 | 19,624 | 32,829 | 52,454 | 5,872 | 12,249 | 18,121 | 25,497 | 45,078 | 70,574 | 63.9 |
| 2053 | 19,974 | 33,872 | 53,846 | 5,984 | 12,634 | 18,618 | 25,958 | 46,506 | 72,464 | 64.2 |
| 2054 | 20,339 | 34,957 | 55,296 | 6,098 | 13,026 | 19,124 | 26,437 | 47,984 | 74,420 | 64.5 |
| 2055 | 20,668 | 36,094 | 56,761 | 6,217 | 13,424 | 19,641 | 26,884 | 49,518 | 76,402 | 64.8 |
| 2056 | 21,000 | 37,247 | 58,247 | 6,336 | 13,832 | 20,168 | 27,336 | 51,080 | 78,415 | 65.1 |
| 2057 | 21,371 | 38,498 | 59,869 | 6,461 | 14,260 | 20,720 | 27,831 | 52,758 | 80,590 | 65.5 |
| 2058 | 21,709 | 39,672 | 61,381 | 6,586 | 14,708 | 21,294 | 28,296 | 54,380 | 82,676 | 65.8 |

1) Excludes any allowance for uncollectible revenue.
2) Fiscal year extends from July 1 through June 30.
3) Monroe Expressway assumed to open on January 1, 2019. Only 6 months of operation are assumed in FY 2019.
4) Includes an assumed ramp-up to full traffic volumes.

### 6.11.3 Bill by Mail Processing Fee Revenue Assumptions

Under current NCTA business rules, Bill by Mail transactions are invoiced on a 35 -day cycle. If a customer does not pay the first invoice for an unpaid toll, a $\$ 6.00$ processing fee is charged with the second invoice. A maximum of $\$ 48.00$ in processing fees may be assessed in a 12 -month period. NCTA assesses an additional $\$ 6.00$ processing fee and a $\$ 25.00$ civil penalty as part of the third invoice if a Bill by Mail invoice remains unpaid. A maximum of $\$ 50.00$ in civil penalties may be assessed in a 12 -month period. It is important to note only the processing fee revenue proceeds are retained by NCTA, thus civil penalties have not been considered as part of this analysis.

Over the course of calendar years 2014 and 2015, approximately 56.9 percent of invoiced Bill by Mail revenue was paid on the first invoice, approximately 15.3 percent was paid on the second invoice, and approximately 15.6 percent was paid on the third or later invoice. Based on this information it was assumed that 30.9 percent of Monroe Expressway Bill by Mail invoices will pay the $\$ 6.00$ processing fee associated with a second invoice. It was also assumed that 15.6 percent of the Monroe Expressway Bill by Mail invoices will pay the additional $\$ 6.00$ processing fee associated with a third or later invoice.

### 6.11.4 Net Toll Revenue

Table 6.14 summarizes the estimated net toll revenue from FY 2019 through FY 2058. Toll revenue estimates are provided for Class 1 and combined Class 2 and 3 vehicles, and by method of payment. The total annual net toll revenue is expected to increase from about $\$ 7.5$ million in FY 2019 to about $\$ 80.8$ million in FY 2058. Toll revenue estimates for the first four fiscal years is dampened to reflect ramp-up. Net toll revenue exceeds gross toll revenue forecasts from FY 2019 through 2039. Net toll revenue is somewhat less than gross toll revenue from 2040 through 2058. This is because fee revenue becomes a smaller component of gross toll revenue over time due to several factors, including: 1) BBM transactions decrease over time as the ETC market share increases, thus the associated fee revenue decreases, and 2) the processing fee is assumed to remain constant throughout the forecast period, while toll rates are assumed to increase annually, therefore, fee revenue does not keep pace with increasing toll rates.

Table 6.14
Estimated Annual Net Toll Revenue For Monroe Expressway (1) (in thousands \$)

| Fiscal <br> Year (2) | Collected Toll Revenue |  |  |  |  |  |  |  |  | Percent ETC | Fee Revenue | Net Toll Revenue |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Class 1 |  |  | Classes 2 and 3 |  |  | All Vehicles |  |  |  |  |  |
|  | BBM | ETC | Total | BBM | ETC | Total | BBM | ETC | Total |  |  |  |
| $2019(3,4)$ | \$2,152 | \$2,545 | \$4,697 | \$590 | \$1,132 | \$1,722 | \$2,742 | \$3,677 | \$6,420 | 57.3 \% | \$1,100 | \$7,519 |
| 2020 (4) | 5,165 | 6,173 | 11,338 | 1,406 | 2,708 | 4,113 | 6,571 | 8,880 | 15,451 | 57.5 | 2,603 | 18,055 |
| 2021 (4) | 6,759 | 8,212 | 14,972 | 1,819 | 3,534 | 5,354 | 8,579 | 11,747 | 20,326 | 57.8 | 3,333 | 23,659 |
| 2022 (4) | 7,887 | 9,739 | 17,626 | 2,100 | 4,113 | 6,213 | 9,987 | 13,852 | 23,839 | 58.1 | 3,805 | 27,644 |
| 2023 | 8,496 | 10,630 | 19,126 | 2,234 | 4,407 | 6,641 | 10,730 | 15,037 | 25,767 | 58.4 | 4,000 | 29,767 |
| 2024 | 8,910 | 11,355 | 20,265 | 2,319 | 4,608 | 6,926 | 11,228 | 15,963 | 27,191 | 58.7 | 4,100 | 31,292 |
| 2025 | 9,276 | 12,103 | 21,379 | 2,395 | 4,800 | 7,195 | 11,671 | 16,903 | 28,574 | 59.2 | 4,176 | 32,750 |
| 2026 | 9,555 | 12,654 | 22,209 | 2,462 | 4,983 | 7,445 | 12,018 | 17,636 | 29,654 | 59.5 | 4,202 | 33,856 |
| 2027 | 9,791 | 13,201 | 22,992 | 2,528 | 5,163 | 7,690 | 12,319 | 18,364 | 30,683 | 59.9 | 4,207 | 34,890 |
| 2028 | 10,026 | 13,842 | 23,868 | 2,593 | 5,347 | 7,940 | 12,619 | 19,190 | 31,808 | 60.3 | 4,212 | 36,020 |
| 2029 | 10,250 | 14,464 | 24,714 | 2,662 | 5,544 | 8,206 | 12,912 | 20,008 | 32,920 | 60.8 | 4,217 | 37,137 |
| 2030 | 10,412 | 14,970 | 25,382 | 2,714 | 5,709 | 8,423 | 13,126 | 20,678 | 33,805 | 61.2 | 4,215 | 38,020 |
| 2031 | 10,561 | 15,467 | 26,029 | 2,770 | 5,890 | 8,660 | 13,332 | 21,357 | 34,689 | 61.6 | 4,210 | 38,898 |
| 2032 | 10,776 | 16,055 | 26,830 | 2,851 | 6,124 | 8,975 | 13,627 | 22,179 | 35,805 | 61.9 | 4,208 | 40,013 |
| 2033 | 10,988 | 16,668 | 27,657 | 2,935 | 6,360 | 9,295 | 13,924 | 23,028 | 36,951 | 62.3 | 4,206 | 41,157 |
| 2034 | 11,193 | 17,341 | 28,533 | 3,023 | 6,605 | 9,627 | 14,215 | 23,945 | 38,161 | 62.7 | 4,204 | 42,365 |
| 2035 | 11,425 | 18,039 | 29,464 | 3,112 | 6,866 | 9,978 | 14,537 | 24,905 | 39,442 | 63.1 | 4,202 | 43,645 |
| 2036 | 11,675 | 18,696 | 30,371 | 3,202 | 7,136 | 10,338 | 14,878 | 25,832 | 40,710 | 63.5 | 4,200 | 44,910 |
| 2037 | 11,926 | 19,390 | 31,315 | 3,295 | 7,420 | 10,714 | 15,220 | 26,809 | 42,030 | 63.8 | 4,198 | 46,228 |
| 2038 | 12,177 | 20,213 | 32,390 | 3,394 | 7,725 | 11,119 | 15,571 | 27,938 | 43,510 | 64.2 | 4,197 | 47,706 |
| 2039 | 12,409 | 20,930 | 33,339 | 3,495 | 8,025 | 11,520 | 15,904 | 28,956 | 44,860 | 64.5 | 4,195 | 49,054 |
| 2040 | 12,651 | 21,723 | 34,374 | 3,598 | 8,336 | 11,934 | 16,249 | 30,059 | 46,308 | 64.9 | 4,193 | 50,501 |
| 2041 | 12,897 | 22,577 | 35,474 | 3,695 | 8,653 | 12,348 | 16,592 | 31,230 | 47,822 | 65.3 | 4,187 | 52,009 |
| 2042 | 13,108 | 23,353 | 36,460 | 3,783 | 8,953 | 12,736 | 16,891 | 32,305 | 49,196 | 65.7 | 4,177 | 53,373 |
| 2043 | 13,323 | 24,229 | 37,552 | 3,872 | 9,259 | 13,131 | 17,195 | 33,488 | 50,682 | 66.1 | 4,166 | 54,849 |
| 2044 | 13,559 | 25,161 | 38,719 | 3,960 | 9,570 | 13,530 | 17,518 | 34,731 | 52,250 | 66.5 | 4,156 | 56,405 |
| 2045 | 13,793 | 26,152 | 39,945 | 4,053 | 9,885 | 13,939 | 17,846 | 36,038 | 53,884 | 66.9 | 4,146 | 58,029 |
| 2046 | 14,058 | 27,081 | 41,139 | 4,141 | 10,205 | 14,346 | 18,198 | 37,286 | 55,484 | 67.2 | 4,136 | 59,620 |
| 2047 | 14,309 | 27,965 | 42,273 | 4,221 | 10,519 | 14,740 | 18,530 | 38,484 | 57,013 | 67.5 | 4,127 | 61,140 |
| 2048 | 14,535 | 28,907 | 43,442 | 4,305 | 10,849 | 15,154 | 18,840 | 39,756 | 58,596 | 67.8 | 4,118 | 62,714 |
| 2049 | 14,799 | 29,869 | 44,669 | 4,389 | 11,191 | 15,581 | 19,188 | 41,061 | 60,249 | 68.2 | 4,109 | 64,358 |
| 2050 | 15,054 | 30,807 | 45,861 | 4,477 | 11,530 | 16,006 | 19,530 | 42,337 | 61,867 | 68.4 | 4,100 | 65,967 |
| 2051 | 15,294 | 31,762 | 47,056 | 4,566 | 11,884 | 16,449 | 19,860 | 43,646 | 63,505 | 68.7 | 4,089 | 67,595 |
| 2052 | 15,557 | 32,829 | 48,387 | 4,655 | 12,249 | 16,904 | 20,213 | 45,078 | 65,291 | 69.0 | 4,077 | 69,368 |
| 2053 | 15,835 | 33,872 | 49,707 | 4,744 | 12,634 | 17,378 | 20,579 | 46,506 | 67,085 | 69.3 | 4,065 | 71,150 |
| 2054 | 16,124 | 34,957 | 51,081 | 4,834 | 13,026 | 17,861 | 20,958 | 47,984 | 68,942 | 69.6 | 4,053 | 72,995 |
| 2055 | 16,385 | 36,094 | 52,478 | 4,928 | 13,424 | 18,352 | 21,313 | 49,518 | 70,831 | 69.9 | 4,041 | 74,872 |
| 2056 | 16,648 | 37,247 | 53,895 | 5,023 | 13,832 | 18,855 | 21,671 | 51,080 | 72,750 | 70.2 | 4,030 | 76,780 |
| 2057 | 16,942 | 38,498 | 55,440 | 5,122 | 14,260 | 19,381 | 22,064 | 52,758 | 74,822 | 70.5 | 4,018 | 78,840 |
| 2058 | 17,210 | 39,672 | 56,882 | 5,221 | 14,708 | 19,930 | 22,432 | 54,380 | 76,812 | 70.8 | 4,006 | 80,818 |

1) Net toll revenue consists of total collected toll revenue and processing fee revenue.
2) Fiscal year extends from July 1 through June 30.
3) Monroe Expressway assumed to open on January 1, 2019. Only 6 months of operation are assumed in FY 2019.
4) Includes an assumed ramp-up to full traffic volumes.

### 6.12 Disclaimer

Current accepted professional practices and procedures were used in the development of these updated traffic and revenue forecasts. However, as with any forecast of the future, it should be understood that there may be differences between forecasted and actual results caused by events and circumstances beyond the control of CDM Smith. In formulating its forecasts, CDM Smith has reasonably relied upon the accuracy and completeness of information provided (both written and oral) by the NCDOT/NCTA and other local and state agencies. CDM Smith also has relied upon the reasonable assurances of some independent parties and is not aware of any facts that would make such information misleading.

CDM Smith has made qualitative judgments related to several key variables in the development and analysis of the traffic and revenue forecasts that must be considered as a whole; therefore, selecting portions of any individual result without consideration of the intent of the whole may create a misleading or incomplete view of the results and the underlying methodologies used to obtain the results. CDM Smith gives no opinion as to the value or merit to partial information extracted from this report.

All forecasts and projections reported herein are based on CDM Smith's experience and judgment and on a review of information obtained from multiple state and local agencies, including NCDOT/NCTA, by an independent third party. These estimates and projections may not be indicative of actual or future values, and are therefore subject to substantial uncertainty. Future developments cannot be predicted with certainty, and may affect the forecasts or projections expressed in this report, such that CDM Smith does not specifically guarantee or warrant any forecasts or projections contained within this report.

While CDM Smith believes that some of the projections or other forward-looking statements contained within the report are based on reasonable assumptions as of the date in the report, such forward looking statements involve risks and uncertainties that may cause actual results to differ materially from the results predicted. Therefore, following the date of this report, CDM Smith will take no responsibility or assume any obligation to advise of changes that may affect its assumptions contained within the report, as they pertain to: socioeconomic and demographic forecasts, proposed residential or commercial land use development projects and/or potential improvements to the regional transportation network.

CDM Smith is not, and has not been, a municipal advisor as defined in Federal law (the Dodd Frank Bill) to NCDOT/NCTA and does not owe a fiduciary duty pursuant to Section 15B of the Exchange Act to NCDOT/NCTA with respect to the information and material contained in this report. CDM Smith is not recommending and has not recommended any action to NCDOT/NCTA. NCDOT/NCTA should discuss the information and material contained in this report with any and all internal and external advisors that it deems appropriate before acting on this information.

## Chapter 7

## Sensitivity Tests

Five tests were conducted to determine the sensitivity of the Base Condition annual transaction and toll revenue forecasts to changes in key study assumptions. The sensitivity tests were conducted for calendar years 2019 and 2040. The results were converted to fiscal year (FY) and are reported for FY 2019 and FY 2040. The following describes the five sensitivity tests:

1. Reduced Economic Growth - trip table growth in the MRM were reduced by 30 percent.
2. Reduced Value of Time - passenger car (Class 1) and truck (Classes 2 and 3) values of time were reduced by 25 percent.
3. Increased ETC Market Share - passenger car and truck ETC market shares were increased by 20 percent.
4. Increased Motor Fuel Prices - motor fuel prices were increased by 50 percent.
5. Reduced Truck Market Share - 25 percent of the forecast truck transactions on the Monroe Expressway were assumed to be passenger cars. Total toll transactions remained unchanged.

Table 7.1 shows the Base Condition forecasts for fiscal year toll transactions, gross toll revenue, and net toll revenue. The results of the five sensitivity tests are also shown, including the difference and percent impact between the sensitivity test forecast and the Base Condition forecast. The percent impact of each sensitivity test compared to the Base Condition net toll revenue forecast is graphically shown in Figure 7.1.

### 7.1 Reduced Economic Growth

The rate of trip table growth was reduced by 30 percent between 2015 and 2040 to simulate slower economic growth than assumed in the MRM model. As a result of the slower growth, toll transactions decreased by 13 percent in FY 2019 and 20 percent in FY 2040. Similarly, gross and net toll revenue decreased by 12 percent in FY 2019 and 18 percent in FY 2040. The percent impacts in 2040 are larger compared to the 2019 percent impacts because of the compounding nature of the reduced annual growth rates.

### 7.2 Reduced Value of Time

Motorist value of time (VOT) is an important factor in the modeling process, as it influences a driver's willingness to pay a toll in order to achieve a time savings by using the toll road. The VOT is based in part on the median household income in each traffic analysis zone in the MRM. Base Condition VOTs were reduced by 25 percent in calendar years 2019 and 2040, resulting in an estimated 11 percent and 10 percent decrease in transactions in FY 2019 and FY 2040, respectively. Annual gross and net toll revenue estimates were also reduced by about 11 and 10 percent in FY 2019 and FY 2040, respectively.

Table 7.1
Summary of Sensitivity Test Results on Fiscal Year Transaction and Toll Revenue Estimates (in thousands)

| Base Condition and Sensitivity Scenarios |  | Annual <br> Toll Transactions |  | Annual Gross <br> Toll Revenue (7) |  | Annual Net <br> Revenue (8) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 2019 (6) | 2040 | 2019 (6) | 2040 | 2019 (6) | 2040 |
|  | Base Condition | 14,324 | 68,799 | \$7,137 | \$50,556 | \$7,519 | \$50,501 |
| Sensitivity Scenarios |  |  |  |  |  |  |  |
| 1 | Overall Economic Growth Reduced by 30\% (1) | 12,422 | 55,326 | \$6,274 | \$41,392 | \$6,610 | \$41,347 |
|  | Difference | $(1,902)$ | $(13,473)$ | (863) | $(9,164)$ | (909) | $(9,154)$ |
|  | Percent Difference | -13.3\% | -19.6\% | -12.1\% | -18.1\% | -12.1\% | -18.1\% |
| 2 | Value of Time Reduced by $25 \%$ (2) | 12,806 | 61,938 | \$6,334 | \$45,408 | \$6,673 | \$45,359 |
|  | Difference | $(1,518)$ | $(6,861)$ | (803) | $(5,148)$ | (846) | $(5,142)$ |
|  | Percent Difference | -10.6\% | -10.0\% | -11.3\% | -10.2\% | -11.3\% | -10.2\% |
| 3 | ETC Market Share Increased by 20\% (3) | 14,753 | 70,854 | \$6,922 | \$48,633 | \$7,214 | \$48,708 |
|  | Difference | 429 | 2,055 | (215) | $(1,923)$ | (305) | $(1,793)$ |
|  | Percent Difference | 3.0\% | 3.0\% | -3.0\% | -3.8\% | -4.1\% | -3.6\% |
| 4 | Fuel Price Increased by 50\% (4) | 13,502 | 64,976 | \$6,809 | \$48,255 | \$7,174 | \$48,203 |
|  | Difference | (822) | $(3,823)$ | (328) | $(2,301)$ | (345) | $(2,298)$ |
|  | Percent Difference | -5.7\% | -5.6\% | -4.6\% | -4.6\% | -4.6\% | -4.6\% |
| 5 | Truck Market Share Reduced by 25\% (5) | 14,324 | 68,799 | \$6,805 | \$48,294 | \$7,226 | \$48,447 |
|  | Difference | 0 | 0 | (332) | $(2,262)$ | (293) | $(2,054)$ |
|  | Percent Difference | 0.0\% | 0.0\% | -4.7\% | -4.5\% | -3.9\% | -4.1\% |
| 1) | 30 percent global reduction in MRM trip table growth from base year 2015 through 2040. |  |  |  |  |  |  |
| 2) | Value of time is reduced by 25 percent for cars and trucks. |  |  |  |  |  |  |
| 3) | Electronic toll collection market share is reduced by 20 percent for cars and trucks. |  |  |  |  |  |  |
| 4) | Fuel price is increased by 50 percent. |  |  |  |  |  |  |
| 5) | Truck transactions forecast for the Monroe Expressway are reduced by 25 percent. Those same transactions are assumed to be car transactions. Total transactions remain unchanged. |  |  |  |  |  |  |
| 6) | Includes a dampening factor (ramp-up) on traffic and revenue. |  |  |  |  |  |  |
| 7) | Total expected toll revenue for all toll transactions, prior to accounting for leakage or fee revenue. |  |  |  |  |  |  |
| 8) | Total collected toll revenue and processing fee revenue. |  |  |  |  |  |  |

### 7.3 Increased ETC Market Share

The Base Condition assumes that Class 1 ETC participation will increase from about 59 percent in calendar year 2019, to about 68 percent in 2040 . Combined Class 2 and 3 ETC participation will increase from about 70 percent in 2019 to 74 percent in 2040. Conversely, the use of VTC is assumed to decrease over the years as ETC increases.

The increased ETC market share test assumes a 20 percent increase in ETC participation for all vehicle classes. In 2019 the Class 1 ETC market share would be 71 percent, reaching 82 percent in 2040. Because of the 35 percent discounted ETC toll rate, compared to the VTC rate, and a larger pool of ETC vehicles, the toll diversion assignment indicates that toll transactions would increase by about 3 percent in both FY 2019 and FY 2040. Annual gross toll revenue decreases by 3 percent in FY 2019 and 4 percent in FY 2040 due to the lower ETC toll rates. Net toll revenue also decreases by about 4 percent in FY 2019 and FY 2040.


### 7.4 Increased Motor Fuel Prices

Motor fuel prices can be volatile, and this test assumes that the Base Condition motor fuel prices increase by 50 percent in each assignment year. This change would result in higher operating costs for the motorist and likely result in reduced travel demand. To reflect the reduced travel demand, the trip tables were reduced by 5 percent. The motor vehicle operating cost was also increased to reflect the increase in motor fuel prices. Under this scenario, annual toll transactions decreased by 6 percent in FY 2019 and FY 2040, and annual gross and net toll revenues decreased by 5 percent in FY 2019 and FY 2040.

### 7.5 Reduced Truck Market Share

Truck toll rates are significantly higher than passenger car toll rates. Class 2 vehicles (those with 3 axles) pay twice the Class 1 ( 2 axle vehicles) toll rates, and Class 3 vehicles (those with 4 or more axles) pay four times the Class 1 toll rates. If the market share of Class 2 and Class 3 vehicles on the Monroe Expressway is reduced and converted to Class 1 vehicles, there would be a negative impact on toll revenue. For the sensitivity test, 25 percent of the Class 2 and 3 vehicles forecast to use the Monroe Expressway were assumed to be Class 1 vehicles. The total number of transactions remain unchanged from the Base Condition. Annual gross toll revenue decreased by 5 percent and net toll revenue decreased by 4 percent.


[^0]:    Note: MRM15v1.1 socioeconomic data for 2010 were derived from the 2010 U.S. Census.

[^1]:    1) Assumes toll rates will be increased annually on January 1

    Note:
    Class 2 tolls will be two times the Class 1 tolls Class 3 tolls will be four times the Class 1 tolls

    ETC tolls receive a 35 percent discount from the BBM tolls

